

International Advanced Research Journal in Science, Engineering and Technology

Vol. 8, Issue 8, August 2021

DOI: 10.17148/IARJSET.2021.8877

Effect of Compacting Temperature on Marshall Properties in 80/100 Grade and PMB Bituminous Concrete Mixes

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ABSTRACT: As we all know Pavement is a structure constructed to provide safe, durable and good riding surface with minimum maintenance. Hence it is very much important to make the pavement functionally and structurally sound throughout its life. The ability to characterize asphalt pavement materials in terms of their fundamental properties is becoming increasingly more important.

The performance of the bituminous pavements mainly depends upon the material property, gradation, type of filler material, binder, mixing and compaction temperature. Therefore, it is essential to study the effect of temperature both during mixing and compacting the material to produce a given mix. The variation in the temperature during mixing and compacting will result in variation in bituminous mix fundamental properties.

This paper will show how the effect of compaction temperature changes in Marshall properties of different Bituminous Concrete mixes such as 80/100 Grade and PMB (Polymer Modified Binder) and also variation in Marshall Properties in soaked condition will show how the pavement react when it comes in contact with water during rain.

Key words: Compaction temperature, PMB, 80/100, stripping, Soaking Index, Marshall Properties, Marshall Stability

1. INTRODUCTION

1.1 General

The design of bituminous mix by Marshall Method involves the Grading of the aggregates and bitumen to produce a mix that will have the adequate qualities and properties. The purpose is to develop a design, by trial means, which will contain optimum amount of bitumen, having adequate voids, satisfactory flow properties and possess a proper combination of stability, durability and flexibility, based on the climatic condition, traffic density and loads that it will carry.

Bituminous mix consists of a combination of aggregate uniformly mixed and coated with bitumen binder. Term "hot mix" comes from mixing of dry aggregate with bitumen heated for proper mixing temperature to attain workability of mix with desired temperature. Polymer modified bitumen is emerging as one of the important construction materials for flexible pavement. Choice of polymers varies from virgin polymer to waste rubber. Generally, the polymers are either blended or dispersed with bitumen and the mix is used as the modified binder.

Once the Aggregate and Asphalt are combined in the mixing plant, the mix will be transported to site and spread with paving machine in loosely compacted layer to uniform, smooth surface. Then the mix will be compacted by heavy roller to produce smooth and well consolidated course.

Compaction is one of major issue in bituminous mix and important criteria in process to produce good quality of hot mix asphalt. There are various instances where surface layer has failed due to carelessness in maintaining proper temperature during mixing and compaction.

Compacting bitumen at too high temperature also may result in hairline cracks and mix displacement and if the bitumen is compacted at too low a temperature, there is a risk that the bonds between the aggregate and the binder will break up, or that the aggregate will be crushed.

Determining the compacting temperature will help to controls bitumen viscosity which affects its ability to coat and provide adequate lubrication for aggregates to slides with each other and pack into dense mass during compaction.

1.2 Objectives of the Present Study

• To determine the effect of compaction temperature on the properties of a compacted bituminous concrete surface course mix.

• To establish the relationships between the fundamental mix properties i.e, Marshall Stability, Flow, Unit Weight, Voids Filled with Bitumen (VFB), Voids ratio (Vv) and compacting temperature.

• To study the effect of compacting temperature on the soaking condition in the laboratory for BC mix.



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1.3 Scope of the Present Study

• Preparation of conventional Marshall Stability test compaction specimens of BC mix using different binders such as 80/100 and PMB for various compacting temperatures of 100°C, 120°C, 140°C, 160°C.

- To determine the mix properties Vv, VFB, Unit Weight for the above specimens.
- To conduct Marshall Stability of Unsoaked specimens for the prepared specimens.

2. LITERATURE REVIEW

2.1 General

There are various instances where surface layer has failed due to carelessness in maintaining proper temperature during mixing and compaction. Bitumen is a thermo plastic material. It means that the consistency of the material changes with change in temperature. Bitumen softens whenever it is heated and becomes more viscous (i.e. high resistance to flow) when cooled. Consistency of bitumen binders generally accepted in India is based on standard tests namely, penetration, ductility and softening point. The physical properties of bitumen mixtures are very much influenced by the rheological properties of the binder. The requirement of bituminous mixtures is to have high compressive strength to resist the deformation caused by the traffic load. It is also desirable for the mixture to have an adequate tensile strength to resist wheel friction and tension caused by contraction. In addition to have sufficient stability to prevent crushing, the bituminous pavements should be sufficiently dense to make it water proof and durable to preserve these qualities. The above properties of bituminous mixtures can be achieved by proper selection of ingredients of the mixture. Even after the proper selection of materials, specifications and proper mix design, the properties of bituminous mixes are governed by other factors also. These factors include time and temperature of heating, Mixing, type of compaction, curing etc.^[5]

2.2 History of Research

Hadley (1970) et al has conducted an extensive laboratory study to find the effect of seven factors on the tensile properties of asphalt material. The result of the studies showed that compaction temperature along with asphalt content, grade of asphalt, and aggregate gradation has significant cause to tensile strength. In this study also the entire result was affected by the compaction temperature and produced strong interaction with other variables.^[12]

Hadley (1978) conducted second laboratory test and founded that reclaiming agent and compaction temperature has certain effect to the significant increase in tensile strength, static and resilient modulus of elasticity. However, as the amount of reclaiming agent increase, the effect of compaction temperature was more important. The viscosity effect seems to be minimal since all samples were heated and mixed at the same temperature of 135°C. This experiment was conducted using recycled asphalt mixtures.

2.3 Effect of Compaction Temperature

The effect of compaction temperature is evaluated during quality control/quality assurance testing. Inaccurate control of compaction temperature could very well occur. It has always been assumed that the compacted density of bituminous concrete is very dependent upon the temperature. Studies have shown that, the percentage of air voids achieved by the Marshall hammer decreased from approximately 10.3 to 7.1.Therefore, the compaction temperature of the prepared mixes was varied to determine whether or not poor control would significantly increase the variability of volumetric properties. The effects of compaction temperature can be subdivided with respect to density and engineering properties. In procedure of pavement construction the compaction is done when temperature reaches 110°C and in laboratory bituminous mix normally compacted when temperature reaches 145°C.

PRESENT INVESTIGATION

3.1 Materials used in the study

A. Aggregates

Granite aggregates available in the quarry near Bangalore was selected for mix design. Aggregate fraction retained on IS sieve 4.75 mm was used as coarse aggregate. Aggregates retained on IS sieve .075mm was used as fine aggregate.

SI No	Type of Aggregates	Specific Gravity
1	Coarse aggregate, 20mm	2.69
2	Coarse aggregate, 12mm	2.68
3	Coarse aggregate, 6mm	2.67
4	Fine aggregates	2.72

Table 3.1 Specific gravity of aggregates used in current study

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Table 3.2 Properties of aggregates used in present study

Sl no	Property	Values	MORT&H Specification
1	Los Angels Abrasion value	26.80%	MAX 40%
2	Aggregate Crushing value	34.66%	MAX 45%
3	Aggregate Impact value	25.68%	MAX 30%
4	Flakiness Index	9.50%	MAX 15%
5	Elongation Index	17.50%	
6	Combined Elongation and Flakiness Indices	21%	MAX 30%
7	Angularity Number	8	0 TO 11

B. Bitumen (Tests on Binders)

Bitumen of grade 80/100 and PMB are used for this study and the physical properties of the bitumen were tested and the results are given in the Table 3.3

SI		Gr		
no	Property	80/100	РМВ	Test Method
1	Penetration (mm)	85	70	IS: 1203-1978
2	Softening Point, °C	47	55	IS: 1205-1978
3	Ductility at 27°C,cm	85	60	IS: 1208-1979
4	Specific Gravity	1.2	1.2	IS: 1202-1980
5	Flash and Fire Point, °C	265	220	IS: 1209-1981

3.2 Aggregate Gradation Obtained for mix

The different sizes of aggregates i.e. 20 mm, 12 mm and 6 mm are selected from the heap and the sieve analysis is done to obtain the individual gradation of these aggregates. Then by Trial and Error method, the desired gradation for BC (Bituminous Concrete) mix is obtained to match the midpoint gradation and is shown in the Table 3.4 **Table 3.4 Aggregate Gradation for Marshall Specimens for BC Mix (As per MoRT&H Specification) and**

Obtained Gradation

IS Sieve (mm)	% passing (specified)	% passing (Mid- Limit)	% passing (obtained)
26.5	100	100	100.00
19	90-100	95	90.16
9.5	56-80	68	65.00
4.75	35-65	50.5	50.29
2.36	23-49	36	31.92
0.3	5-19	12	13.02
0.075	2-8	5	4.62



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3.3 Marshal Stability Test Conducted on Bituminous mix

The properties like stability and bulk density of any bituminous mix are mainly dependent on gradation of aggregate, binder content, the type of compaction, method adopted for compaction, and the temperature during compaction.



Breaking Head



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Fig 3.2: Apparatus for Marshal Stability Test



Aggregate Mix







Marshall Stability Testing Machine Fig 3.3: Preparation of the specimen for Marshall Stability Test

As per MoRT&H, Compaction temperature for different grade of Bitumen is 100°C to 120°C and as per IRC SP-53-2002 Compaction temperature for Polymer Bitumen 115°C to 135°C. Therefore for the present investigation, BC mix using 80/100 grade and PMB-70 at various compaction temperatures were tried to investigate the effect on Marshall Properties of the mix.

As per the above standards, the minimum compaction temperature for 80/100 and PMB mix is 100°C and 115°C. For the present study, the compaction temperature were varied from 100°C to 160°C with an interval of 20°C i.e. 100°C, 120°C, 140°C & 160°C.

ANALYSIS OF DATA OBTAINED

4.1 **Determination of OBC (Optimum Binder Content)**

Marshall Stability specimens were prepared with 80/100 plain bitumen by, varying the binder content from 4.2% to 7.2% by an increment of 0.5%. Three specimens were prepared for each binder content.

Marshall Stability test was conducted and properties like stability, flow, bulk density, volume of voids and voids filled with bitumen were found for 80/100 plain bitumen. Using these properties, optimum binder content is calculated corresponding for maximum stability, maximum bulk density and 4% volume of voids.

a	able 4.1 Marshall Stability properties for various bitumen content									
	Bitumen	Marshall	Flow,	Bulk density,	Voids	Voids in Mineral	Voids filled with			
	%	Stability,kg	mm	g/cc	ratio%	Aggregate,%	Bitumen,%			
				C						
L										
	4.20	1250	2.60	2.217	13.32	22.67	40.26			
	4.70	1350	3.40	2.286	9.31	19.85	53.08			

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5.20	1450	3.75	2.324	7.60	18.66	63.49
5.70	1325	4.32	2.310	7.01	19.92	64.80
6.20	1310	5.40	2.302	6.91	20.90	66.95
6.70	1260	5.80	2.351	3.40	18.84	81.96
7.20	1190	6.20	2.352	2.87	19.47	85.24

Table 4.2 Results obtained from graphs:

1	Maximum Stability, Kg	1450	At bitumen content 5.2%	Average = 5.2%
2	Maximum Bulk density, g/cc	2.324	At bitumen content 5.2%	C
3	Percent air voids	4%	At bitumen content 5.2%	





Fig 4.1: Marshall Properties of Bituminous Concrete Mix to determine OBC

4.2 Marshall properties of Bituminous concrete mix

Marshall Stability test were conducted by varying compacting temperature from 100°C to 160°C by an increment of 20°C using optimum bitumen content 5.2% and test results are presented in Table 4.3 showing Marshall stability and Flow value for 80/100 and PMB mix both soaked and unsoaked condition. Comparison of Marshall properties v/s Compaction temperature are tabulated in Table 4.4 and Fig 4.3

Table 4.3 Marshall Stability values and Flow values of BC mix, 80/100 and PMB:



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Temperature		Marshall st	tability (kg)		Flow(mm)			
(°C)	80/100	80/100 (kg) PMB (kg) 80/100		PMB (kg)		00	PMB	
	Unsoaked	soaked	Unsoaked	soaked	Unsoaked	soaked	Unsoaked	soaked
100	624	444	686	495	4.3	7.5	3.0	5.0
120	768	564	878	664	4.0	7.3	2.6	4.5
140	1032	768	1136	878	3.5	7.0	2.3	3.8
160	1200	900	1418	1114	3.3	6.5	2.0	3.5



Fig 4.2 : Comparison of Marshall Stability & Flow Parameters v/s Compaction Temperature at 1000C,1200C,1400C and 1600C for PMB and 80/100 BC mix (soaked and unsoaked)

Temp	Vv (%)	VFB	(%)	Vb ((%)	Gm (g/cc)	VMA	(%)	Soaking	g Index
°C	80/100	PMB	80/100	PMB	80/100	PMB	80/100	PMB	80/100	PMB	80/100	PMB
100	7.67	6.23	60.65	65.78	11.81	11.97	2.32	2.35	19.47	18.19	0.71	0.72
120	6.77	5.44	63.76	68.94	11.91	12.06	2.34	2.36	18.67	17.49	0.73	0.76
140	6.32	4.72	65.42	72.03	11.96	12.14	2.35	2.38	18.28	16.85	0.74	0.77
160	5.79	4.03	67.47	75.18	12.02	12.22	2.36	2.40	17.81	16.25	0.75	0.79

Table 4.4 Basic Marshall Properties Of BC Mix, 80/100 and PMB:

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5. DISCUSSIONS AND CONCLUSIONS

The results analysed from the laboratory investigations on 80/100 and PMB binders at different temperature 100°C to 160°C are discussed with respect to their mix properties and susceptibility for stripping through soaking conditions as below.

5.2 Effect of Compaction temperature on various Marshall Properties.

Marshall	Change in property w.r.t	Remarks
Property	increase in temp	
Voids ratio(%)	a. decreases linearly from 7.67 to 5.79 - 80/100 mix	This decrease is due to lubricating effect of BC keeping viscosity of the binder suitable for compaction.
	b. decreases linearly from6.23 to 4.03 - PMB mix	
Voids Filled Bitumen (%)	 a. increases linearly from 60.65 to 67.47 - 80/100 mix b. increases linearly from 65.78 to 75.18 - PMB mix 	This increase is due to lubricating effect of binder, which increase the workability of the mix and improves the compaction.

5.1

General



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Bulk Density	 a. increases linearly from 2.32 to 2.36 - 80/100 mix b. increases linearly from 2.35 to 2.40 - PMB mix 	This is because temperature decreases viscosity and made compaction easy.
Marshall Flow (mm)	 a. decreases linearly from 4.3 to 3.3 - 80/100 mix b. decreases linearly from 3.0 to 2.0 - PMB mix 	 a. The flow required as per MoRTH i.e. 2-4 is achieved in the range of temperature 120°C to 140°C for 80/100 mix b. The flow required as per IRC-SP-53-2002 i.e. 2.5-4 is achieved in the range of temperature 140°C to 160°C for PMB mix
Marshall Stability(kg) (unsoaked)	 a. increases linearly from 624 to 1200 - 80/100 mix b. increases linearly from 686 to 1418 - PMB mix 	 a. The stability required as per MoRTH i.e. 900kg is achieved at the temperature 130°C for 80/100 mix b. The stability required as per IRC-SP-53-2002 i.e. 1000kg is achieved at the temperature 131.46°C for PMB mix
Marshall Stability(kg) (soaked)	 a. increases linearly from 444 to 900 - 80/100 mix b. increases linearly from 495 to 1114 - PMB mix 	This increase is due to the more adhesive forces caused by the decrease in the viscosity of the BC to reach the good workable and compaction condition

5.3 CONCLUSIONS

1. Bituminous concrete mix grade-II considered for the study is greatly influenced by compacting temperature w.r.t mix properties like Vv, VFB, Flow, Stability, and Bulk Density.

2. Marshall Stability increases with increase in compacting temperature. Marshall stability required as per MoRTH for BC mix is found to be achieved at temperature range of 120°C to 140°C for 80/100 grade and range of 130°C to 150°C for PMB grade as per IRC-SP-53-2002.

3. Marshall Flow decreases with the increase of compaction temperature because it increases the bitumen coating of the aggregate, and the filling of pores with binder.

4. Soaking Index which represents the tendency of the mix to stripping, increases with the increase in

temperature which is a natural phenomenon where the temperature acts as catalyst whereas the soaking index changes less in PMB compared to 80/100 bitumen.

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