

Free Vibrations of Simply Supported Rectangular Mindlin Plate at Higher Modes Using Coupled Displacement Field Method

K Krishna Bhaskar¹, K Meera saheb², V Kalyana Manohar³

Assistant Professor, Department of Mechanical Engineering, University College of Engineering Kakinada, India^{1,3}

Professor, Department of Mechanical Engineering, University College of Engineering Kakinada, India²

Abstract: An accurate closed form solution was developed to evaluate the free vibration behaviour of thin and moderately thick isotropic rectangular plates with simply supported boundary condition for all the edges. A novel methodology known as coupled displacement field (CDF) method, proposed by the author was used to study the free vibration of the rectangular plate. Here an admissible trial function which satisfies the boundary conditions was assumed for one of the variables (say total rotations) and another variable which is the lateral displacement field is derived in terms of the initial variable by using the coupling equations, where the two independent variables become dependent on one another. The proposed CDF method makes use of the energy formulation and results in only half the number of undetermined coefficients when compared with the conventional Rayleigh-Ritz method. The vibration problem gets simplified significantly due to the reduction in the number of undetermined coefficients. The plate problem was also solved in Rayleigh-Ritz method to show the efficacy and simplicity of the CDF method. The Primary focus was given to the effect of aspect ratio and slenderness ratio on the non-dimensional frequency parameter at higher modes. The numerical results obtained by the present methodology are validated with Rayleigh-Ritz method and results available in the existing literature where ever possible. The analysis of the plate problem is based on Mindlin plate theory and the effect of shear deformation, as well as rotary inertia, were included.

Keywords: coupled displacement field, coupling equation, large amplitude vibrations, moderately thick plates.

I. INTRODUCTION

The complex structures in various fields of engineering are made up of simple structural members like beams (slender or short), plates (thin or moderately thick) and shells (thin or moderately thick). If these structural members are subjected to severe dynamic environment they vibrate. One of the essential consideration in design of these structures is to evaluation of free vibration. The conventional energy methods provide an effective means in evaluating the fundamental frequency parameters of these structural members and the results obtained by these approaches will act as an upper limit for comparison.

Accuracy of assumed trial function define the efficiency of energy methods. For thin plates, shear deformation and rotary inertia effects can be neglected. Therefore only one admissible trail function is enough for the analysis. But for moderately thick plates, these effects have to be considered and they are introduced by choosing separate admissible trial functions for the total rotations and lateral displacement [1]. The vibration study on such plates have been reported in by Leissa [2, 3].

The coupled displacement field (CDF) concept was implemented in the finite element (FE) analysis and was reported in the open literature. Free vibration analysis of moderately thick beams of uniform thickness by CDF method was successfully demonstrated [4]. In this study, the present methodology was extended to moderately thick rectangular plates for evaluating free vibration behaviour of uniform shear deformable rectangular plates.

For thin plates the fundamental frequency parameter was evaluated in Refs. [5,6] with several configurations and boundary conditions. S. H. Hashemi *et al.* [7] used dimensionless equation of motion to solve the problem based on first order shear deformation theory of plates to analyze the transverse vibration behaviour of thick rectangular plates. Many researchers have successfully employed different shear deformation plate theories to solve problems related to free vibration analysis of plates [8-14]. Non linear analysis of plates was evaluated by using finite element method [19-27].

For the analysis of thin and moderately thick rectangular plates, shear deformation and rotary inertia effects are considered by choosing separate admissible functions for both total rotations and lateral displacement fields. Thus, if n

term admissible trial function is chosen for the total rotation, another n term admissible trial function for the lateral displacement result in, and the vibration problem becomes $2n$ unknown coefficients. The vibration problem analyzed by classical Rayleigh Ritz method contain $2n$ homogeneous simultaneous equations which are to be solved which was explained in the present paper.

In the present methodology, for the analysis of moderately thick rectangular plates, if an n term admissible trial function is chosen for the total rotation, the admissible trail function for lateral displacement is derived in terms of total rotation by using coupling equation. Both the functions contain same undetermined coefficients.

The coupling equation used to couple the independent fields was derived from static equilibrium equations. The complexity of vibration problem will be reduced as the number of undetermined coefficients reduced to half when compared to classical Rayleigh Ritz method. The practicality of this method is verified by solving the vibration problem considering uniform and isotropic moderately thick rectangular plates with all edges simply supported condition for different slenderness and aspect ratios.

II. FIRST ORDER SHEAR DEFORMATION THEORY OF PLATES

In first order shear deformation plate theory (or FSDT), also known as the Mindlin plate theory, the displacements u, v and w are given by

$$u(x, y, z) = z\theta_x(x, y). \quad (1)$$

$$v(x, y, z) = z\theta_y(x, y). \quad (2)$$

$$w(x, y, z) = w(x, y). \quad (3)$$

where u is inplane displacement along x direction, v is inplane displacement along y direction and, w is transverse displacement along z direction, θ_x and θ_y denote rotations about the y and x axes respectively.

In this theory, shear correction factor k was introduced to correct the discrepancy between the actual transverse shear stress distribution and that computed using the kinematic relations of FSDT. A value of $k=5/6$, the widely used value of the shear correction factor, is used in the present study.

III. RAYLEIGH-RITZ METHOD FOR MODERATELY THICK RECTANGULAR PLATES

In the conventional Rayleigh Ritz method separate functions for total rotations (θ_x and θ_y) and transverse displacement (w) are to be assumed in the analysis of moderately thick rectangular plates. The functions for total rotations and transverse displacement that satisfy the kinematic boundary conditions can be assumed as

$$w = \sum_{i=1}^n \alpha_{1i} f_{w_i}(x, y). \quad (4)$$

$$\theta_x = \sum_{i=1}^n \alpha_{2i} f_{w_{xi}}(x, y). \quad (5)$$

$$\theta_y = \sum_{i=1}^n \alpha_{3i} f_{w_{yi}}(x, y). \quad (6)$$

For rectangular plate the undetermined coefficients of total rotations are the same. Hence there are only two undetermined coefficients, i.e, α_{1i} , α_{2i} . To know the efficacy of the CDF method initially free vibration problems are solved using Rayleigh Ritz method. In the present Rayleigh Ritz method, for better understanding, a single term admissible function is taken for θ_x , θ_y and w .

Expressions for strain energy (U) and kinetic energy (T) for moderately thick rectangular plates are given as

$$\begin{aligned}
 U &= \frac{D}{2} \int_0^a \int_0^b \left\{ \left(\frac{\partial \theta_x}{\partial x} \right)^2 + \left(\frac{\partial \theta_y}{\partial y} \right)^2 + 2\nu \frac{\partial \theta_x}{\partial x} \frac{\partial \theta_y}{\partial y} + 2(1-\nu) \frac{\partial \theta_x}{\partial y} \frac{\partial \theta_y}{\partial x} \right\} dx dy \\
 &+ \frac{kGh}{2} \int_0^a \int_0^b \left\{ \left(\frac{dw}{dx} + \theta_x \right)^2 + \left(\frac{dw}{dy} + \theta_y \right)^2 \right\} dx dy \\
 T &= \frac{\rho h \omega^2}{2} \int_0^a \int_0^b \left[w^2 + \frac{h^2}{12} (\theta_x^2 + \theta_y^2) \right] dx dy. \quad (7)
 \end{aligned}$$

where D is the flexural rigidity, G is rigidity modulus, h is thickness of the plate and ν is the poisson's ratio. By substituting the assumed functions, we derive the expressions for strain energy and kinetic energy.

The minimization of Lagrangian, with respect to undetermined coefficient α_1 and α_2 in case of single term admissible functions are given as:

$$\frac{\partial(U - T)}{\partial \alpha_1} = 0. \quad (9)$$

and

$$\frac{\partial(U - T)}{\partial \alpha_2} = 0. \quad (10)$$

By solving the above two homogeneous equations, the non-dimensional fundamental frequency parameter (λ) is obtained.

IV. EXPRESSION FOR NON-DIMENSIONAL FUNDAMENTAL FREQUENCY PARAMETER OF MODERATELY THICK RECTANGULAR PLATE USING RAYLEIGH RITZ METHOD

Single term admissible functions for total rotations (θ_x, θ_y) and transverse displacement (w) for the assumed fundamental mode are taken as

$$\theta_x = \alpha \frac{m\pi}{a} \cos \frac{m\pi x}{a} \sin \frac{n\pi y}{b}. \quad (11)$$

$$\theta_y = \alpha \frac{n\pi}{b} \sin \frac{m\pi x}{a} \cos \frac{n\pi y}{b}. \quad (12)$$

$$w = \beta \sin \frac{m\pi x}{a} \sin \frac{n\pi y}{b}. \quad (13)$$

substituting Eqs. (11-13) in Eq. (7) and Eq. (8) and after integration, the expression for strain energy and kinetic energy are obtained as

$$U = \frac{D}{2} \frac{ab}{4} \left\{ \beta^2 (p^2 + q^2)^2 + \left(\frac{k6(1-\nu)}{h^2} (\alpha + \beta)^2 (p^2 + q^2) \right) \right\}. \quad (14)$$

$$T = \frac{\rho h \omega^2}{2} \frac{ab}{4} \left(\alpha^2 + \frac{h^2}{12} \beta^2 (p^2 + q^2) \right). \quad (15)$$

where $p = \frac{m\pi}{a}$, $q = \frac{n\pi}{b}$ and $b = s a$

By minimizing the Lagrangian with respect to undetermined coefficient α and β , the following homogeneous quadratic equations are obtained.

$$\frac{D}{8} ab \left[\frac{12k(1-\nu)}{h^2} (\alpha + \beta) (p^2 + q^2) \right] - \left[\frac{\rho h \omega_L^2 ab}{4} \alpha \right] = 0. \quad (16)$$

$$\frac{D}{8} ab \left[2(p^2 + q^2)^2 + \frac{12k(1-\nu)}{h^2} (\alpha + \beta) (p^2 + q^2) \right] - \left[\frac{\rho h^3 \omega_L^2 ab}{48} (p^2 + q^2) \beta \right] = 0 \quad (17)$$

$$\lambda^2 \left(\frac{h}{a} \right)^4 - \left(6 \left(2\pi^2 \left(\frac{h}{a} \right)^2 \left(\frac{m^2 s^2 + n^2}{s^2} \right) \right) + k(1-\nu) \left(12 + \left(\frac{h}{a} \right)^2 \left(\frac{m^2 s^2 + n^2}{s^2} \right) \pi^2 \right) \right) \lambda + 4k(1-\nu) \left(\frac{m^2 s^2 + n^2}{s^2} \right) \pi^4 = 0 \quad (18)$$

$$\lambda = \frac{\rho h \omega_L^2 a^4}{D} = \frac{G - \sqrt{H}}{2 \left(\frac{h}{a} \right)^4} \quad (19)$$

where $G = \left(6 \left(2\pi^2 \left(\frac{h}{a} \right)^2 \left(\frac{m^2 s^2 + n^2}{s^2} \right) \right) + k(1-\nu) \left(12 + \left(\frac{h}{a} \right)^2 \left(\frac{m^2 s^2 + n^2}{s^2} \right) \pi^2 \right) \right)$ and

$$H = 36 \left(2\pi^2 \left(\frac{h}{a} \right)^2 \left(\frac{m^2 s^2 + n^2}{s^2} \right) \right) + k(1-\nu) \left(12 + \left(\frac{h}{a} \right)^2 \left(\frac{m^2 s^2 + n^2}{s^2} \right) \pi^2 \right)^2 - 4k(1-\nu) \left(\frac{h}{a} \right)^4 \left(\frac{m^2 s^2 + n^2}{s^2} \right) \pi^4$$

V. COUPLED DISPLACEMENT FIELD (CDF) METHOD FOR MODERATELY THICK RECTANGULAR PLATES

In this section the detailed procedure for evaluating fundamental linear frequency parameter of uniform simply supported moderately thick rectangular plate based on present methodology is discussed in detail. The admissible trial functions for θ_x and θ_y are assumed in the functional form. They satisfy the boundary conditions and symmetric conditions for the fundamental mode.

$$\theta_x = \alpha \frac{m\pi}{a} \cos \frac{m\pi x}{a} \sin \frac{n\pi y}{b} \quad (20)$$

$$\theta_y = \alpha \frac{n\pi}{b} \sin \frac{m\pi x}{a} \cos \frac{n\pi y}{b} \quad (21)$$

VI. COUPLING EQUATIONS FOR MODERATELY THICK RECTANGULAR PLATES

The coupling equations for evaluating the transverse displacement distribution w in x and y directions are given as

$$\frac{dw}{dx} = -\theta_x + \frac{h^2}{3.5} \left[\frac{\partial^2 \theta_x}{\partial x^2} + \nu \frac{\partial^2 \theta_y}{\partial y \partial x} \right] + \frac{h^2}{10} \left[\frac{\partial^2 \theta_x}{\partial y^2} + \frac{\partial^2 \theta_y}{\partial y \partial x} \right]. \quad (22)$$

$$\frac{dw}{dy} = -\theta_y + \frac{h^2}{3.5} \left[\frac{\partial^2 \theta_y}{\partial y^2} + \nu \frac{\partial^2 \theta_x}{\partial y \partial x} \right] + \frac{h^2}{10} \left[\frac{\partial^2 \theta_y}{\partial x^2} + \frac{\partial^2 \theta_x}{\partial y \partial x} \right]. \quad (23)$$

substituting Eq. (20) and Eq. (21) in Eq. (22) and Eq. (23) and after integration and evaluation of the constant of integration we get

$$w = -\alpha \sin px \sin qy \left[1 + \frac{h^2}{3.5} (p^2 + \nu q^2) + \frac{h^2}{5} (q^2) \right]. \quad (24)$$

$$U = \frac{D}{2} \alpha^2 ab \left\{ (p^2 + q^2)^2 + \left(\frac{k6(1-\nu)}{h^2} (p^2(A-1)^2 + q^2(B-1)^2) \right) \right\}. \quad (25)$$

where $A = \left[1 + \frac{h^2}{3.5} (p^2 + \nu q^2) + \frac{h^2}{5} (q^2) \right]$ and $B = \left[1 + \frac{h^2}{3.5} (q^2 + \nu p^2) + \frac{h^2}{5} (p^2) \right]$

Substituting Eq. (20), Eq. (21) and Eq. (24) in Eq. (8) and after simplification we find kinetic energy as

$$T = \frac{\rho h \omega_L^2}{2} \alpha^2 \frac{ab}{4} \left(A^2 + \frac{h^2}{12} (p^2 + q^2) \right). \quad (26)$$

By minimizing the Lagrangian with respect to undetermined coefficient α we get the non-dimensional fundamental frequency parameter.

$$\frac{\partial(U-T)}{\partial \alpha} = 0. \quad (27)$$

$$\lambda = \frac{\rho h \omega_L^2 a^4}{D} = \frac{\pi^4 \left[\left(m^2 + \left(\frac{n}{s} \right)^2 \right)^2 + k6(1-\nu) \pi^2 \left(m^2 A_1^2 + \left(\frac{n B_1}{s} \right)^2 \right) \left(\frac{h}{a} \right)^2 \right]}{\left[\left(1 + \pi^2 A_1 \left(\frac{h}{a} \right)^2 \right)^2 + \frac{\pi^2}{12} \left(m^2 + \left(\frac{n}{s} \right)^2 \right) \left(\frac{h}{a} \right)^2 \right]}. \quad (28)$$

where $A_1 = \frac{1}{3.5} \left(m^2 + \nu \frac{n^2}{s^2} \right) + \frac{n^2}{5s^2}$ and $B_1 = \frac{1}{3.5} \left(\frac{n^2}{s^2} + \nu \right) + \frac{m^2}{5}$.

VII. LARGE AMPLITUDE FREE VIBRATIONS

By applying law of conservation of energy the total energy is constant at any instant of time neglecting damping. In order to evaluate the large amplitude free vibrations of plates, the work done due to the stretching of the central plane of the plate has to be derived. The stretching of the central plane in x direction causes tension and its expression can be given as

$$T_x = \frac{Eh}{2a} \int_0^a \left(\frac{dw(x)}{dx} \right)^2 dx. \quad (29)$$

$$T_x = \frac{Eh}{4} \alpha^2 \frac{\pi^2}{a^2}. \quad (30)$$

Shear flexible terms are not involved in the expression of tension because the tension in the plate is only caused by stretching of the mid plane due to bending and not due to shear. Similarly the tension in y direction can be given as

$$T_x = \frac{Eh}{2a} \int_0^a \left(\frac{dw(x)}{dx} \right)^2 dx. \quad (31)$$

$$T_x = \frac{Eh}{4} \alpha^2 \frac{\pi^2}{a^2}. \quad (32)$$

The expression for work done is given as

$$W = \frac{1}{4} \int_0^a \int_0^a \left\{ T_x \left(\frac{dw}{dx} \right)^2 + T_y \left(\frac{dw}{dy} \right)^2 \right\} dx dy. \quad (33)$$

Substituting Eq. (22), Eq. (23), Eq. (30) and Eq. (32) in Eq. (33) and after simplification we get

$$W = \alpha^4 \frac{Eh}{64} a^2 s \left(p^4 A^2 + q^4 B^2 \right). \quad (34)$$

By applying total energy principle

$$U + T + W = constant. \quad (35)$$

Substituting Eq. (25), Eq. (26), Eq. (34) and $D = Eh^3/12(1-\nu^2)$ in Eq. (35) and after simplification

$$\dot{z}^2 + \eta_1 z^2 + \eta_2 z^4 = constant. \quad (36)$$

where

$$\alpha_1 = \frac{\rho h \omega_L^2 a^4}{D} = \frac{\pi^4 \left[\left(m^2 + \left(\frac{n}{s} \right)^2 \right)^2 + k 6(1-\nu) \pi^2 \left(m^2 A_1^2 + \left(\frac{n B_1}{s} \right)^2 \right) \left(\frac{h}{a} \right)^2 \right]}{\left[\left(1 + \pi^2 A_1 \left(\frac{h}{a} \right)^2 \right)^2 + \frac{\pi^2}{12} \left(m^2 + \left(\frac{n}{s} \right)^2 \right) \left(\frac{h}{a} \right)^2 \right]}. \quad (37)$$

$$\alpha_2 = \frac{\rho h \omega_{NL}^2 a^4}{D} = \frac{\pi^4 \frac{3}{2} (1-\nu^2) \left[m^4 \left(1 + A_1 \pi^2 \left(\frac{h}{a} \right)^2 \right)^2 + \frac{n^4}{s^4} \left(1 + B_1 \frac{\pi^2}{s^2} \left(\frac{h}{a} \right)^2 \right)^2 \right]}{\left[\left(1 + \pi^2 A_1 \left(\frac{h}{a} \right)^2 \right)^2 + \frac{\pi^2}{12} \left(m^2 + \left(\frac{n}{s} \right)^2 \right) \left(\frac{h}{a} \right)^2 \right]} \quad (38)$$

By following the harmonic balance method the expression for frequency ratio can be obtained as

$$\left[\frac{\omega_{NL}}{\omega_L} \right]^2 = 1 + 1.5 \left(\frac{\alpha_2}{\alpha_1} \right) \left(\frac{\alpha_m}{h} \right)^2 \quad (39)$$

$$\frac{\alpha_2}{\alpha_1} = \frac{\frac{3}{2} (1-\nu^2) \left[m^4 \left(1 + A_1 \pi^2 \left(\frac{h}{a} \right)^2 \right)^2 + \frac{n^4}{z^4} \left(1 + B_1 \frac{\pi^2}{z^2} \left(\frac{h}{a} \right)^2 \right)^2 \right]}{\left[\left(m^2 + \left(\frac{n}{z} \right)^2 \right)^2 + k 6 (1-\nu) \pi^2 \left(m^2 A_1^2 + \left(\frac{n B_1}{z} \right)^2 \right) \left(\frac{h}{a} \right)^2 \right]} \quad (40)$$

Eq. (39) gives the expression for frequency ratio in terms of thickness ratio (h/a) and maximum amplitude ratio (α_m/h).

VIII. RESULTS AND DISCUSSION

To show the simplicity, efficacy and ease of application of the proposed CDF method, initially linear free vibration problem of moderately thick rectangular plate with all edges simply supported boundary condition (Fig. 1) at higher modes is solved by using Rayleigh Ritz and CDF methods. Single term trigonometric admissible functions are used in these two methods to study the influence of plate thickness ratio and aspect ratio on the frequency parameter at fundamental mode and higher modes. Numerical results in the form of frequency parameter at different modes for a plate thickness ratio of 0.001 as a function of different aspect ratios is given in Table 1. For the purpose of validation the results obtained by the present CDF, RR methods along with other researchers are also included in Table 1 and they are in good agreement. It is observed from Table 1 that frequency parameter increases with mode number. It is also observed that frequency parameter decreases with increase of aspect ratio for a particular given mode sequence. Similar trend for the frequency parameter has been observed for other plate thickness ratios such as 0.05, 0.1, 0.15 and 0.2 which are given respectively in Table 2-5.

It is found from Table 1-5 that around 11.5 % reduction in frequency parameter has been observed in moderately thick plate (Plate thickness ratio=0.2) when compared to thin plate (Plate thickness ratio=0.001) for an aspect ratio 1 for fundamental mode and a variation of 55 % for mode sequence 44. Around 6.5 % reduction in frequency parameter has been observed in moderately thick plate (Plate thickness ratio=0.2) when compared to thin plate (Plate thickness ratio=0.001) for an aspect ratio of 10 for fundamental mode and a variation of 43 % for mode sequence of 44.

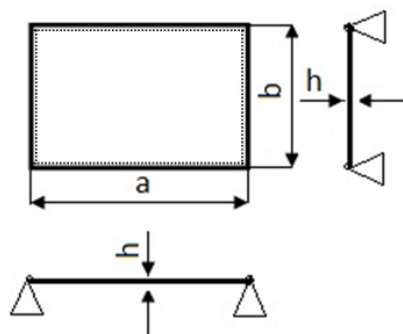


Fig.1. Uniform simply supported moderately thick rectangular plates

where u is inplane displacement along x direction, v is inplane displacement along y direction and, w is transverse displacement along z direction, θ_x and θ_y denote rotations about the y and x axes respectively. In this theory, shear correction factor k was introduced to correct the discrepancy between the actual transverse shear stress distribution and that computed using the kinematic relations of FSDT. A value of $k=5/6$, the widely used value of the shear correction factor, is used in the present study.

IX. FIRST ORDER SHEAR DEFORMATION THEORY OF PLATES

In first order shear deformation plate theory (or FSDT), also known as the Mindlin plate theory, the displacements u, v and w are given by

$$u(x, y, z) = z\theta_x(x, y). \quad (1)$$

$$v(x, y, z) = z\theta_y(x, y). \quad (2)$$

$$w(x, y, z) = w(x, y). \quad (3)$$

For the sake of comparison and validation of proposed method, the same results obtained by the finite element method (FEM) [20] and of other researchers [25, 26, 27] are included in table 6, table 7 for aspect ratio 1 and 2 respectively for fundamental mode and the agreement found is very good. The present results in terms time period ratio (T_{NL}/T_L) as a function of plate thickness ratio, aspect ratio and mode sequence are given respectively in Table 8-12 for different amplitude ratios such as 0.2, 0.4, 0.6, 0.8 and 1. The time period ratio is evaluated by taking reciprocal of frequency ratio given in Eq. (39). Table 8 shows the variation of time period ratio with different plate thickness ratios, aspect ratios and mode sequence for an amplitude ratio of 0.2.

Results show that there is around 2 % to 3 % (variation) decrease in time period for thin plate when nonlinearity is considered and this variation increases to around 3 % to 8 % for moderately thick plate for a thickness ratio of 0.2 and aspect ratio 1. With the increase in aspect ratio the time period ratio decreases. The decrease in time period considering nonlinearity is around 2 % to 4 % when the aspect ratio is increased from 1 to 10 for thin plate. As the plate thickness ratio is increased to 0.2 this variation increases to 3 % to 5% in fundamental mode and reaches 8% to 10% for 44 mode. Table 9-12 shows the variation of time period ratio for maximum amplitude ratios 0.4, 0.6, 0.8 1 respectively.

The observations of results from Table 9-12 are similar to table 8 but the variation is predominant with the increase in amplitude ratio. Results in Table 12 for maximum amplitude ratio 1 shows that there is around 30 % to 40 % (variation) decrease in time period for thin plate when nonlinearity is considered and this variation increases to around 40 % to 60 % for moderately thick plate for a thickness ratio of 0.2 and aspect ratio 1. This gives a clear indication from Table 8-12 that time period ratio decreases with increase of aspect ratio, amplitude ratio and plate thickness ratio. The effect of amplitude ratio is predominant on time period ratio. Shear deformation and rotary inertia contribution has been observed in all above tables for the given all edges simply supported plate.

X. CONCLUSION

The concept of coupled displacement field method (CDF) is successfully applied to study the free vibrations of moderately thick rectangular plates with all edges simply supported boundary condition. Elegant and closed form expressions for non-dimensional frequency parameter was derived in terms of plate thickness ratio (h/a) aspect ratio and mode number.

The closed form expression was obtained for the time period ratio as a function of maximum amplitude ratio a_m/h and plate thickness ratio h/a . The non-dimensional frequency parameter and time period results obtained by the present method are very accurate when compared to the published results.

It indicates that the CDF method can be applied for thin and moderately thick problems. From the present study it is evident that CDF method predicts the vibration behavior of rectangular plates accurately and even minimizes the computational effort.

Table 1. Linear frequency parameter ($\sqrt{\lambda}$) for thickness ratio 0.001

| b/a | Ref | Mode Number | | | | | | | | | | | | | | | |
|-----|-----|-------------|---------|---------|----------|---------|---------|----------|----------|---------|----------|----------|----------|----------|----------|----------|----------|
| | | 11 | 12 | 13 | 14 | 21 | 22 | 23 | 24 | 31 | 32 | 33 | 34 | 41 | 42 | 43 | 44 |
| 1 | CDF | 19.7391 | 49.3476 | 98.6942 | 167.7781 | 49.3476 | 78.9557 | 128.3018 | 197.3849 | 98.6942 | 128.3018 | 177.6471 | 246.7289 | 167.7781 | 197.3849 | 246.7289 | 315.8089 |
| | RR | 19.7391 | 49.3476 | 98.6942 | 167.7780 | 49.3476 | 78.9557 | 128.3020 | 197.3850 | 98.6942 | 128.3200 | 177.6470 | 246.7290 | 167.7780 | 197.3850 | 246.7290 | 315.8090 |
| | 15 | 19.7482 | 49.3433 | 98.5957 | 167.4018 | 49.3433 | 78.9058 | 128.1059 | 196.8359 | 98.5957 | 128.1059 | 177.2171 | 245.8236 | 167.4018 | 196.8359 | 245.8236 | 314.2592 |
| | 16 | 19.7551 | 49.3878 | 98.7745 | 167.9134 | 49.3878 | 79.0204 | 128.4062 | -- | 98.7745 | 128.4062 | -- | -- | 167.9134 | -- | -- | -- |
| | 3 | 19.7392 | 49.3480 | 98.6960 | -- | 49.3480 | 78.9568 | 128.3049 | -- | 98.6960 | 128.3049 | -- | -- | 167.7833 | -- | -- | -- |
| | 17 | 19.7391 | 49.3475 | 98.6943 | -- | 49.3475 | 78.9556 | 128.3019 | -- | 98.6943 | 128.3019 | -- | -- | -- | -- | -- | -- |
| 1.5 | CDF | 14.2561 | 27.4154 | 49.3476 | 80.0523 | 43.8646 | 57.0238 | 78.9557 | 109.6601 | 93.2113 | 106.3703 | 128.3018 | 159.0056 | 162.2953 | 175.4540 | 197.3849 | 228.0879 |
| | RR | 14.2561 | 27.4154 | 49.3476 | 80.0523 | 43.8646 | 57.0238 | 78.9557 | 109.6600 | 93.2113 | 106.3700 | 128.3020 | 159.0060 | 162.2950 | 175.4540 | 197.3850 | 228.0880 |
| | 15 | 14.2642 | 27.4240 | 49.3433 | 80.0003 | 43.8652 | 57.0103 | 78.9058 | 109.5292 | 93.1285 | 106.2499 | 128.1059 | 158.6740 | 161.9474 | 175.0361 | 196.8359 | 227.3279 |
| | 16 | 14.2681 | 27.4398 | 49.3868 | 80.1168 | 43.9008 | 57.0695 | 79.0194 | 109.7485 | 93.2866 | 106.4563 | -- | -- | -- | -- | -- | -- |
| | 3 | 14.2561 | 27.4156 | 49.3480 | 80.0535 | 43.8649 | 57.0244 | 78.9568 | -- | 93.2129 | 106.3724 | -- | -- | -- | -- | -- | -- |
| 2 | CDF | 12.3370 | 19.7391 | 32.0760 | 49.3476 | 41.9455 | 49.3476 | 61.6843 | 78.9557 | 91.2923 | 98.6942 | 111.0308 | 128.3018 | 160.3763 | 167.7781 | 180.1143 | 197.3849 |
| | RR | 12.3370 | 19.7391 | 32.0760 | 49.3476 | 41.9455 | 49.3476 | 61.6843 | 78.9557 | 91.2923 | 98.6942 | 111.0310 | 128.3020 | 160.3760 | 167.7780 | 180.1140 | 197.3850 |
| | 15 | 12.3440 | 19.7482 | 32.0833 | 49.3433 | 41.9470 | 49.3433 | 61.6646 | 78.9058 | 91.2142 | 98.5957 | 110.8953 | 128.1059 | 160.0381 | 167.4018 | 179.6697 | 196.8359 |
| | 16 | 12.3469 | 19.7541 | 32.1020 | 49.3868 | 41.9796 | 49.3868 | 61.7337 | 79.0194 | 91.3673 | -- | -- | -- | -- | -- | -- | -- |
| 2.5 | CDF | 11.4487 | 16.1861 | 24.0817 | 35.1356 | 41.0572 | 45.7946 | 53.6901 | 64.7438 | 90.4041 | 95.1413 | 103.0367 | 114.0902 | 159.4881 | 164.2252 | 172.1204 | 183.1737 |
| | RR | 11.4487 | 16.1861 | 24.0817 | 35.1356 | 41.0572 | 45.7946 | 53.6901 | 64.7438 | 90.4041 | 95.1413 | 103.0370 | 114.0900 | 159.4880 | 164.2250 | 172.1200 | 183.1740 |
| | 15 | 11.4560 | 16.1942 | 24.0904 | 35.1414 | 41.0600 | 45.7933 | 53.6805 | 64.7197 | 90.3282 | 95.0527 | 102.9261 | 113.9455 | 159.1540 | 163.8676 | 171.7202 | 182.7120 |
| | 16 | 11.4599 | 16.2012 | 24.1032 | 35.1641 | 41.0906 | 45.8318 | 53.7339 | 64.7967 | -- | -- | -- | -- | -- | -- | -- | -- |
| | 3 | 11.4487 | 16.1862 | 24.0818 | 35.1358 | 41.0576 | 45.7950 | 53.6906 | -- | -- | -- | -- | -- | -- | -- | -- | -- |
| 3 | CDF | 10.9662 | 14.2561 | 19.7391 | 27.4154 | 40.5747 | 43.8646 | 49.3476 | 57.0238 | 89.9216 | 93.2113 | 98.6942 | 106.3703 | 159.0056 | 162.2953 | 167.7781 | 175.4540 |
| | RR | 10.9662 | 14.2561 | 19.7391 | 27.4154 | 40.5747 | 43.8646 | 49.3476 | 57.0238 | 89.9216 | 93.2113 | 98.6942 | 106.3700 | 159.0060 | 162.2950 | 167.7780 | 175.4540 |
| | 15 | 10.9730 | 14.2642 | 19.7482 | 27.4240 | 40.5770 | 43.8652 | 49.3433 | 57.0103 | 89.8462 | 93.1285 | 98.5957 | 106.2499 | 158.6740 | 161.9474 | 167.4018 | 175.0361 |
| 5 | CDF | 10.2644 | 11.4487 | 13.4226 | 16.1861 | 39.8729 | 41.0572 | 43.0311 | 45.7946 | 89.2198 | 90.4041 | 92.3779 | 95.1413 | 158.3038 | 159.4881 | 161.4619 | 164.2252 |
| | RR | 10.2644 | 11.4487 | 13.4226 | 16.1861 | 39.8729 | 41.0573 | 43.0312 | 45.7945 | 89.2197 | 90.4041 | 92.3779 | 95.1413 | 158.3038 | 159.4882 | 161.4621 | 164.2251 |
| | 15 | 10.2707 | 11.4560 | 13.4305 | 16.1942 | 39.8757 | 41.0600 | 43.0316 | 45.7933 | 89.1469 | 90.3282 | 92.2968 | 95.0527 | 157.9756 | 159.1540 | 161.1177 | 163.8676 |
| 10 | CDF | 9.9683 | 10.2644 | 10.7578 | 11.4487 | 39.5768 | 39.8729 | 40.3664 | 41.0572 | 88.9237 | 89.2198 | 89.7132 | 90.4041 | 158.0078 | 158.3038 | 158.7973 | 159.4881 |
| | RR | 9.9683 | 10.2644 | 10.7578 | 11.4487 | 39.5769 | 39.8729 | 40.3663 | 41.0573 | 88.9237 | 89.2197 | 89.7132 | 90.4041 | 158.0079 | 158.3038 | 158.7974 | 159.4882 |
| | 15 | 9.9744 | 10.2707 | 10.7646 | 11.4560 | 39.5803 | 39.8757 | 40.3696 | 41.0600 | 88.8515 | 89.1469 | 89.6388 | 90.3282 | 157.6813 | 157.9756 | 158.4665 | 159.1 |

Table 2. Linear frequency parameter ($\sqrt{\lambda}$) for thickness ratio 0.05

| b/a | Ref | Mode Number | | | | | | | | | | | | | | | |
|-----|-----|-------------|---------|---------|----------|---------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|
| | | 11 | 12 | 13 | 14 | 21 | 22 | 23 | 24 | 31 | 32 | 33 | 34 | 41 | 42 | 43 | 44 |
| 1 | CDF | 19.5624 | 48.2697 | 94.5465 | 156.3867 | 48.2697 | 76.2603 | 121.4466 | 181.9428 | 94.5465 | 121.4466 | 164.9652 | 223.393 | 156.3867 | 181.9428 | 223.393 | 279.2334 |
| | RR | 19.5624 | 48.2695 | 94.5454 | 156.3797 | 48.2695 | 76.25988 | 121.4438 | 181.9308 | 94.54544 | 121.4438 | 164.9567 | 223.3676 | 156.3797 | 181.9308 | 223.3676 | 279.1775 |
| 1.5 | CDF | 14.1635 | 27.0767 | 48.2697 | 77.2839 | 43.0091 | 55.5933 | 76.2603 | 104.5818 | 89.496 | 101.5803 | 121.4466 | 148.7099 | 151.5945 | 163.0641 | 181.9428 | 207.8956 |
| | RR | 14.1635 | 27.0767 | 48.2695 | 77.2834 | 43.0090 | 55.59317 | 76.2599 | 104.5801 | 89.4952 | 101.579 | 121.4438 | 148.7041 | 151.5883 | 163.0561 | 181.9308 | 207.8759 |
| 2 | CDF | 12.2675 | 19.5624 | 31.6142 | 48.2697 | 41.162 | 48.2697 | 60.0167 | 76.2603 | 87.7231 | 94.5465 | 105.8302 | 121.4466 | 149.9127 | 156.3867 | 167.1003 | 181.9428 |
| | RR | 12.2675 | 19.5624 | 31.6141 | 48.2695 | 41.162 | 48.26956 | 60.0165 | 76.25988 | 87.72235 | 94.54544 | 105.8286 | 121.4438 | 149.9066 | 156.3797 | 167.0916 | 181.9308 |
| 2.5 | CDF | 11.3889 | 16.0669 | 23.8197 | 34.5828 | 40.3061 | 44.8636 | 52.4186 | 62.9111 | 86.9016 | 91.2763 | 98.5308 | 108.6117 | 149.1334 | 153.2835 | 160.1687 | 169.7428 |
| | RR | 11.3888 | 16.0669 | 23.8196 | 34.5828 | 40.3061 | 44.86357 | 52.4185 | 62.9109 | 86.90086 | 91.27541 | 98.52964 | 108.6099 | 149.1278 | 153.2772 | 160.1612 | 169.7333 |
| 3 | CDF | 10.9113 | 14.1635 | 19.5624 | 27.0767 | 39.8408 | 43.0091 | 48.2697 | 55.5933 | 86.4551 | 89.496 | 94.5465 | 101.5803 | 148.7099 | 151.5945 | 156.3867 | 163.0641 |
| | RR | 10.9113 | 14.1635 | 19.5624 | 27.0767 | 39.8408 | 43.00907 | 48.2696 | 55.5932 | 86.45438 | 89.4952 | 94.54544 | 101.579 | 148.7041 | 151.5883 | 156.3797 | 163.0561 |
| 5 | CDF | 10.2162 | 11.3889 | 13.3405 | 16.0669 | 39.1638 | 40.3061 | 42.2073 | 44.8636 | 85.8054 | 86.9016 | 88.7264 | 91.2763 | 148.0937 | 149.1334 | 150.8644 | 153.2835 |
| | RR | 10.2162 | 11.3888 | 13.3405 | 16.0669 | 39.1637 | 40.30608 | 42.2072 | 44.8636 | 85.80466 | 86.90086 | 88.72559 | 91.27541 | 148.0881 | 149.1278 | 150.8582 | 153.2772 |
| 10 | CDF | 9.9228 | 10.2162 | 10.705 | 11.3889 | 38.878 | 39.1638 | 39.6399 | 40.3061 | 85.5311 | 85.8054 | 86.2623 | 86.9016 | 147.8336 | 148.0937 | 148.527 | 149.1334 |
| | RR | 9.92285 | 10.2162 | 10.7049 | 11.3888 | 38.8780 | 39.16376 | 39.6399 | 40.3061 | 85.5304 | 85.80466 | 86.26152 | 86.90086 | 147.8279 | 148.0881 | 148.5214 | 149.1278 |

Table 3. Linear frequency parameter () for thickness ratio 0.1

| b/a | Ref | Mode Number | | | | | | | | | | | | | | | |
|-----|-----|-------------|---------|---------|----------|---------|---------|----------|----------|---------|----------|----------|----------|----------|----------|----------|----------|
| | | 11 | 12 | 13 | 14 | 21 | 22 | 23 | 24 | 31 | 32 | 33 | 34 | 41 | 42 | 43 | 44 |
| 1 | CDF | 19.0651 | 45.4857 | 85.0656 | 133.7399 | 45.4857 | 69.8084 | 106.7420 | 152.7853 | 85.0656 | 106.7420 | 140.1940 | 182.6167 | 133.7399 | 152.7853 | 182.6167 | 221.0848 |
| | RR | 19.0650 | 45.4826 | 85.0380 | 133.6211 | 45.4826 | 69.7943 | 106.6836 | 152.6086 | 85.0380 | 106.6836 | 140.0571 | 182.3239 | 133.6211 | 152.6086 | 182.3239 | 220.6003 |
| | 15 | 19.0805 | 45.5237 | 85.1326 | 133.8230 | 45.5237 | 69.8649 | 106.8188 | 152.8689 | 85.1326 | 106.8188 | 140.2780 | 182.6981 | 133.8230 | 152.8689 | 182.6981 | 221.1653 |
| | 16 | 19.0805 | 45.5197 | 85.1070 | 133.7292 | 45.5197 | 69.8501 | 106.7694 | -- | -- | 106.7694 | -- | -- | 133.7292 | -- | -- | -- |
| | 18 | 19.0736 | 45.4861 | 84.9963 | -- | 45.4861 | 69.7750 | 106.5985 | -- | 84.9963 | 106.5985 | -- | -- | -- | -- | -- | -- |
| | 7 | 19.0649 | 45.4826 | 85.0380 | 133.6212 | 45.4826 | 69.7943 | 106.6836 | -- | 85.0380 | 106.6836 | -- | -- | -- | -- | -- | -- |
| | 17 | 19.0653 | 45.4869 | 85.0646 | -- | 45.4869 | 69.8093 | 106.7350 | -- | 85.0646 | 106.7350 | -- | -- | -- | -- | -- | -- |
| 1.5 | CDF | 13.8983 | 26.1455 | 45.4857 | 70.6747 | 40.7660 | 51.9739 | 69.8084 | 93.2527 | 80.8969 | 90.8170 | 106.7420 | 127.9089 | 130.1062 | 138.7692 | 152.7853 | 171.6043 |
| | RR | 13.8983 | 26.1451 | 45.4826 | 70.6601 | 40.7640 | 51.9690 | 69.7943 | 93.2151 | 80.8737 | 90.7827 | 106.6836 | 127.8053 | 129.9969 | 138.6366 | 152.6086 | 171.3578 |
| | 15 | 13.9096 | 26.1666 | 45.5237 | 70.7322 | 40.8002 | 52.0172 | 69.8649 | 93.3241 | 80.9613 | 90.8873 | 106.8188 | 127.9913 | 130.1891 | 138.8527 | 152.8689 | 171.6867 |
| | 16 | 13.9096 | 26.1666 | 45.5197 | 70.7173 | 40.7972 | 52.0112 | 69.8501 | 93.2905 | 80.9386 | 90.8557 | -- | -- | -- | -- | -- | -- |
| | 18 | 13.9076 | 26.1558 | 45.4861 | 70.6541 | 40.7647 | 51.9658 | 69.7750 | -- | 80.8379 | -- | -- | -- | -- | -- | -- | -- |
| 2 | CDF | 12.0674 | 19.0651 | 30.3616 | 45.4857 | 39.0967 | 45.4857 | 55.8483 | 69.8084 | 79.4256 | 85.0656 | 94.2625 | 106.7420 | 128.8260 | 133.7399 | 141.7905 | 152.7853 |
| | RR | 12.0674 | 19.0650 | 30.3610 | 45.4826 | 39.0950 | 45.4826 | 55.8419 | 69.7943 | 79.4038 | 85.0380 | 94.2236 | 106.6836 | 128.7202 | 133.6211 | 141.6489 | 152.6086 |
| | 15 | 12.0773 | 19.0805 | 30.3863 | 45.5237 | 39.1289 | 45.5237 | 55.8941 | 69.8649 | 79.4886 | 85.1326 | 94.3346 | 106.8188 | 128.9090 | 133.8230 | 141.8742 | 152.8689 |
| | 16 | 12.0773 | 19.0805 | 30.3853 | 45.5197 | 39.1260 | 45.5197 | 55.8872 | 69.8511 | 79.4679 | -- | -- | -- | -- | -- | -- | -- |
| 2.5 | CDF | 11.2159 | 15.7278 | 23.0918 | 33.0970 | 38.3209 | 42.4357 | 49.1728 | 58.3660 | 78.7424 | 82.3702 | 88.3311 | 96.5058 | 128.2320 | 131.3893 | 136.5932 | 143.7611 |
| | RR | 11.2159 | 15.7277 | 23.0916 | 33.0961 | 38.3193 | 42.4334 | 49.1688 | 58.3585 | 78.7212 | 82.3455 | 88.2998 | 96.4637 | 128.1277 | 131.2768 | 136.4668 | 143.6137 |
| | 15 | 11.2248 | 15.7409 | 23.1105 | 33.1244 | 38.3526 | 42.4705 | 49.2139 | 58.4139 | 78.8051 | 82.4351 | 88.4001 | 96.5787 | 128.3143 | 131.4722 | 136.6767 | 143.8448 |
| | 16 | 11.2248 | 15.7409 | 23.1105 | 33.1224 | 38.3506 | 42.4675 | 49.2080 | 58.4060 | -- | -- | -- | -- | -- | -- | -- | -- |
| 3 | CDF | 10.7523 | 13.8983 | 19.0651 | 26.1455 | 37.8986 | 40.7660 | 45.4857 | 51.9739 | 78.3707 | 80.8969 | 85.0656 | 90.8170 | 127.9089 | 130.1062 | 133.7399 | 138.7692 |
| | RR | 10.7523 | 13.8983 | 19.0650 | 26.1451 | 37.8971 | 40.7640 | 45.4826 | 51.9690 | 78.3499 | 80.8737 | 85.0380 | 90.7827 | 127.8053 | 129.9969 | 133.6211 | 138.6366 |
| | 15 | 10.7606 | 13.9096 | 19.0805 | 26.1666 | 37.9298 | 40.8002 | 45.5237 | 52.0172 | 78.4337 | 80.9613 | 85.1326 | 90.8873 | 127.9913 | 130.1891 | 133.8230 | 138.8527 |
| 5 | CDF | 10.0765 | 11.2159 | 13.1047 | 15.7278 | 37.2834 | 38.3209 | 40.0422 | 42.4357 | 77.8293 | 78.7424 | 80.2588 | 82.3702 | 127.4384 | 128.2320 | 129.5507 | 131.3893 |
| | RR | 10.0765 | 11.2159 | 13.1046 | 15.7277 | 37.2819 | 38.3193 | 40.0402 | 42.4334 | 77.8089 | 78.7212 | 80.2362 | 82.3455 | 127.3362 | 128.1277 | 129.4430 | 131.2768 |
| | 15 | 10.0850 | 11.2248 | 13.1154 | 15.7409 | 37.3144 | 38.3526 | 40.0752 | 42.4705 | 77.8914 | 78.8051 | 80.3223 | 82.4351 | 127.5212 | 128.3143 | 129.6340 | 131.4722 |
| 10 | CDF | 9.7909 | 10.0765 | 10.5518 | 11.2159 | 37.0234 | 37.2834 | 37.7161 | 38.3209 | 77.6006 | 77.8293 | 78.2101 | 78.7424 | 127.2398 | 127.4384 | 127.7693 | 128.2320 |
| | RR | 9.7909 | 10.0765 | 10.5518 | 11.2159 | 37.0220 | 37.2819 | 37.7146 | 38.3193 | 77.5805 | 77.8089 | 78.1893 | 78.7212 | 127.1381 | 127.3362 | 127.6660 | 128.1277 |
| | 15 | 9.7985 | 10.0850 | 10.5601 | 11.2248 | 37.0537 | 37.3144 | 37.7471 | 38.3526 | 77.6632 | 77.8914 | 78.2727 | 78.8051 | 127.3226 | 127.5212 | 127.8521 | 128.3143 |

Table 4. Linear frequency parameter ($\sqrt{\lambda}$) for thickness ratio 0.15

| b/a | Ref | Mode Number | | | | | | | | | | | | | | | |
|-----|-----|-------------|---------|---------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|
| | | 11 | 12 | 13 | 14 | 21 | 22 | 23 | 24 | 31 | 32 | 33 | 34 | 41 | 42 | 43 | 44 |
| 1 | CDF | 18.3295 | 41.8944 | 74.6564 | 112.3571 | 41.8944 | 62.3084 | 91.7247 | 126.5732 | 74.6564 | 91.7247 | 117.2031 | 148.3837 | 112.3571 | 126.5732 | 148.3837 | 175.8471 |
| | RR | 18.3285 | 41.8771 | 74.5533 | 112.0589 | 41.87708 | 62.2476 | 91.5442 | 126.1828 | 74.55327 | 91.5442 | 116.8743 | 147.8421 | 112.0589 | 126.1828 | 147.8421 | 175.112 |
| 1.5 | CDF | 13.4941 | 24.8214 | 41.8944 | 63.0180 | 37.8069 | 47.4409 | 62.3084 | 81.163 | 71.3127 | 79.2353 | 91.7247 | 107.9521 | 109.6151 | 116.1359 | 126.5732 | 140.391 |
| | RR | 13.49378 | 24.8186 | 41.8771 | 62.95506 | 37.79471 | 47.41498 | 62.24757 | 81.03277 | 71.22226 | 79.11346 | 91.5442 | 107.681 | 109.3339 | 115.8136 | 126.1828 | 139.9057 |
| 2 | CDF | 11.7589 | 18.3295 | 28.6205 | 41.8944 | 36.3498 | 41.8944 | 50.7157 | 62.3084 | 70.1274 | 74.6564 | 81.9602 | 91.7247 | 108.6466 | 112.3571 | 118.3972 | 126.5732 |
| | RR | 11.75874 | 18.3285 | 28.6158 | 41.87708 | 36.3391 | 41.87708 | 50.68353 | 62.24757 | 70.04113 | 74.55327 | 81.82652 | 91.5442 | 108.3711 | 112.0589 | 118.0606 | 126.1828 |
| 2.5 | CDF | 10.9479 | 15.2163 | 22.0396 | 31.0606 | 35.6706 | 39.2583 | 45.0563 | 52.8297 | 69.576 | 72.4969 | 77.261 | 83.7274 | 108.1968 | 110.5845 | 114.5032 | 119.8686 |
| | RR | 10.94774 | 15.2158 | 22.0377 | 31.05439 | 35.66048 | 39.24436 | 45.03432 | 52.79309 | 69.49173 | 72.402 | 77.14739 | 83.5857 | 107.924 | 110.2973 | 114.1915 | 119.5224 |
| 3 | CDF | 10.5052 | 13.4941 | 18.3295 | 24.8214 | 35.3003 | 37.8069 | 41.8944 | 47.4409 | 69.2758 | 71.3127 | 74.6564 | 79.2353 | 107.9521 | 109.6151 | 112.3571 | 116.1359 |
| | RR | 10.50505 | 13.4938 | 18.3285 | 24.81862 | 35.29051 | 37.79471 | 41.87708 | 47.41498 | 69.19256 | 71.22226 | 74.55327 | 79.11346 | 107.681 | 109.3339 | 112.0589 | 115.8136 |
| 5 | CDF | 9.8585 | 10.9479 | 12.7433 | 15.2163 | 34.76 | 35.6706 | 37.1759 | 39.2583 | 68.8382 | 69.576 | 70.7989 | 72.4969 | 107.5955 | 108.1968 | 109.1951 | 110.5845 |
| | RR | 9.8584 | 10.9477 | 12.7431 | 15.21581 | 34.75083 | 35.66048 | 37.16423 | 39.24436 | 68.75645 | 69.49173 | 70.71032 | 72.402 | 107.3266 | 107.924 | 108.9165 | 110.2973 |
| 10 | CDF | 9.5847 | 9.8585 | 10.3135 | 10.9479 | 34.5315 | 34.7600 | 35.1401 | 35.6706 | 68.6532 | 68.8382 | 69.146 | 69.576 | 107.4449 | 107.5955 | 107.8463 | 108.1968 |
| | RR | 9.5846 | 9.8584 | 10.3134 | 10.94774 | 34.52246 | 34.75083 | 35.13047 | 35.66048 | 68.57215 | 68.75645 | 69.06323 | 69.49173 | 107.1765 | 107.3266 | 107.5756 | 107.924 |

Table 5. Linear frequency parameter ($\sqrt{\lambda}$) for thickness ratio 0.2

| b/a | Ref | Mode Number | | | | | | | | | | | | | | | |
|-----|-----|-------------|---------|---------|---------|---------|---------|---------|----------|---------|---------|---------|----------|---------|----------|----------|----------|
| | | 11 | 12 | 13 | 14 | 21 | 22 | 23 | 24 | 31 | 32 | 33 | 34 | 41 | 42 | 43 | 44 |
| 1 | CDF | 17.4521 | 38.1963 | 65.3264 | 95.1531 | 38.1963 | 55.2712 | 78.9699 | 106.1494 | 65.3264 | 78.9699 | 98.9140 | 122.8308 | 95.1531 | 106.1494 | 122.8308 | 143.5898 |
| | RR | 17.4486 | 38.1522 | 65.1453 | 94.7658 | 38.1522 | 55.1501 | 78.6970 | 105.6863 | 65.1453 | 78.6970 | 98.5004 | 122.2608 | 94.7658 | 105.6863 | 122.2608 | 142.9066 |
| | 15 | 17.4665 | 38.2172 | 65.3519 | 95.2719 | 38.2172 | 55.2916 | 79.0194 | 106.3476 | 65.3519 | 79.0194 | 99.0570 | 123.2096 | 95.2719 | 106.3476 | 123.2096 | 144.3031 |
| | 16 | 17.4625 | 38.1827 | 65.1978 | 94.8423 | 38.1827 | 55.1948 | 78.7606 | -- | 65.1978 | 78.7606 | -- | -- | 94.8423 | -- | -- | -- |
| | 7 | 17.4485 | 38.1521 | 65.1452 | 94.7658 | 38.1521 | 55.1500 | 78.6969 | -- | 65.1452 | 78.6969 | -- | -- | 94.7658 | -- | -- | -- |
| | 17 | 17.4523 | 38.1883 | 65.3135 | -- | 38.1883 | 55.2543 | 78.9865 | -- | 65.3135 | 78.9865 | -- | -- | -- | -- | -- | -- |
| 1.5 | CDF | 12.9935 | 23.3132 | 38.1963 | 55.8539 | 34.6923 | 42.9011 | 55.2712 | 70.5591 | 62.6206 | 69.0132 | 78.9699 | 91.7221 | 93.0188 | 98.0870 | 106.1494 | 116.7411 |
| | RR | 12.9923 | 23.3038 | 38.1522 | 55.7295 | 34.6591 | 42.8395 | 55.1501 | 70.3437 | 62.4565 | 68.8081 | 78.6970 | 91.3591 | 92.6466 | 97.6791 | 105.6863 | 116.2084 |
| | 15 | 13.0038 | 23.3308 | 38.2172 | 55.8743 | 34.7137 | 42.9219 | 55.2916 | 70.5909 | 62.6434 | 69.0421 | 79.0194 | 91.8217 | 93.1256 | 98.2243 | 106.3476 | 117.0450 |
| | 16 | 13.0028 | 23.3229 | 38.1827 | 55.7746 | 34.6870 | 42.8735 | 55.1948 | 70.4003 | 62.5071 | 68.8633 | -- | -- | -- | -- | -- | -- |
| 2 | CDF | 11.3714 | 17.4521 | 26.6855 | 38.1963 | 33.4350 | 38.1963 | 45.6542 | 55.2712 | 61.6585 | 65.3264 | 71.1974 | 78.9699 | 92.2639 | 95.1531 | 99.8387 | 106.1494 |
| | RR | 11.3707 | 17.4486 | 26.6709 | 38.1522 | 33.4054 | 38.1522 | 45.5809 | 55.1501 | 61.5002 | 65.1453 | 70.9778 | 78.6970 | 91.8970 | 94.7658 | 99.4187 | 105.6863 |
| | 15 | 11.3809 | 17.4665 | 26.7049 | 38.2172 | 33.4563 | 38.2172 | 45.6748 | 55.2916 | 61.6814 | 65.3519 | 71.2300 | 79.0194 | 92.3660 | 95.2719 | 99.9875 | 106.3476 |
| | 16 | 11.3799 | 17.4625 | 26.6921 | 38.1827 | 33.4325 | 38.1827 | 45.6175 | 55.1948 | 61.5500 | -- | -- | -- | -- | -- | -- | -- |
| 2.5 | CDF | 10.6089 | 14.5915 | 20.8178 | 28.8317 | 32.8473 | 35.9403 | 40.8851 | 47.4224 | 61.2104 | 63.5802 | 67.4261 | 72.6103 | 91.9131 | 93.7739 | 96.8204 | 100.9770 |
| | RR | 10.6083 | 14.5897 | 20.8113 | 28.8129 | 32.8192 | 35.9033 | 40.8312 | 47.3412 | 61.0549 | 63.4101 | 67.2314 | 72.3812 | 91.5487 | 93.3963 | 96.4214 | 100.5490 |
| | 15 | 10.6174 | 14.6040 | 20.8337 | 28.8513 | 32.8685 | 35.9612 | 40.9059 | 47.4429 | 61.2329 | 63.6035 | 67.4538 | 72.6454 | 92.0133 | 93.8841 | 96.9492 | 101.1343 |
| | 16 | 10.6164 | 14.6010 | 20.8278 | 28.8365 | 32.8458 | 35.9326 | 40.8644 | 47.3796 | -- | -- | -- | -- | -- | -- | -- | -- |
| 3 | CDF | 10.1915 | 12.9935 | 17.4521 | 23.3132 | 32.5265 | 34.6923 | 38.1963 | 42.9011 | 60.9663 | 62.6206 | 65.3264 | 69.0132 | 91.7221 | 93.0188 | 95.1531 | 98.0870 |
| | RR | 10.1910 | 12.9923 | 17.4486 | 23.3038 | 32.4992 | 34.6591 | 38.1522 | 42.8395 | 60.8123 | 62.4565 | 65.1453 | 68.8081 | 91.3591 | 92.6466 | 94.7658 | 97.6791 |
| | 15 | 10.1996 | 13.0038 | 17.4665 | 23.3308 | 32.5475 | 34.7137 | 38.2172 | 42.9219 | 60.9880 | 62.6434 | 65.3519 | 69.0421 | 91.8217 | 93.1256 | 95.2719 | 98.2243 |
| 5 | CDF | 9.5802 | 10.6089 | 12.2932 | 14.5915 | 32.0580 | 32.8473 | 34.1483 | 35.9403 | 60.6103 | 61.2104 | 62.2038 | 63.5802 | 91.4439 | 91.9131 | 92.6915 | 93.7739 |
| | RR | 9.5798 | 10.6083 | 12.2922 | 14.5897 | 32.0319 | 32.8192 | 34.1167 | 35.9033 | 60.4583 | 61.0549 | 62.0422 | 63.4101 | 91.0828 | 91.5487 | 92.3216 | 93.3963 |
| | 15 | 9.5881 | 10.6174 | 12.3035 | 14.6040 | 32.0783 | 32.8685 | 34.1694 | 35.9612 | 60.6324 | 61.2329 | 62.2266 | 63.6035 | 91.5422 | 92.0133 | 92.7966 | 93.8841 |
| 10 | CDF | 9.3208 | 9.5802 | 10.0105 | 10.6089 | 31.8596 | 32.0580 | 32.3877 | 32.8473 | 60.4597 | 60.6103 | 60.8607 | 61.2104 | 91.3264 | 91.4439 | 91.6396 | 91.9131 |
| | RR | 9.3205 | 9.5798 | 10.0101 | 10.6083 | 31.8341 | 32.0319 | 32.3608 | 32.8192 | 60.3087 | 60.4583 | 60.7072 | 61.0549 | 90.9661 | 91.0828 | 91.2771 | 91.5487 |
| | 15 | 9.3284 | 9.5881 | 10.0188 | 10.6174 | 31.8798 | 32.0783 | 32.4082 | 32.8685 | 60.4812 | 60.6324 | 60.8823 | 61.2329 | 91.4236 | 91.5422 | 91.7387 | 92.0133 |

Table 6. T_M/T_L values of fundamental mode for aspect ratio 1

| h/a | α_m / h | | | | | | | | | |
|-------|----------------|--|--------|--|--------|--|--------|--|--------|--|
| | 0.2 | | 0.4 | | 0.6 | | 0.8 | | 1.0 | |
| | CDF | FEM [19] | CDF | FEM [19] | CDF | FEM [19] | CDF | FEM [19] | CDF | FEM [19] |
| 0.001 | 0.9801 | 0.9818 0.9821* 0.9809\$ 0.9783# | 0.9270 | 0.9327 0.9338* 0.9297\$ 0.9210# | 0.8548 | 0.8652 0.8673* 0.8602\$ 0.8451# | 0.7773 | 0.7916 0.7943* 0.7853\$ 0.7653# | 0.7029 | 0.7199 0.7233* 0.7131\$ 0.6901# |
| 0.05 | 0.9799 | 0.9807 | 0.9260 | 0.9290 | 0.8532 | 0.8586 | 0.7751 | 0.7827 | 0.7005 | 0.7098 |
| 0.1 | 0.9790 | 0.9788 | 0.9233 | 0.9226 | 0.8484 | 0.8475 | 0.7687 | 0.7681 | 0.6931 | 0.6931 |
| 0.15 | 0.9777 | 0.9766 | 0.9188 | 0.9153 | 0.8406 | 0.8350 | 0.7585 | 0.7520 | 0.6814 | 0.6752 |
| 0.2 | 0.9758 | 0.9740 | 0.9126 | 0.9069 | 0.8300 | 0.8212 | 0.7447 | 0.7347 | 0.6660 | 0.6562 |

Values are taken from * [27], \$[26], #[25]

Table 7. T_M/T_L values of fundamental mode for aspect ratio 2

| h/a | α_m / h | | | | | | | | | |
|-------|----------------|----------|--------|----------|--------|----------|--------|----------|--------|----------|
| | 0.2 | | 0.4 | | 0.6 | | 0.8 | | 1.0 | |
| | CDF | FEM [19] | CDF | FEM [19] | CDF | FEM [19] | CDF | FEM [19] | CDF | FEM [19] |
| 0.001 | 0.9733 | 0.9768 | 0.9043 | 0.9161 | 0.8162 | 0.8367 | 0.7272 | 0.7550 | 0.6465 | 0.6795 |
| 0.05 | 0.9731 | 0.9761 | 0.9037 | 0.9140 | 0.8151 | 0.8332 | 0.7258 | 0.7506 | 0.6450 | 0.6747 |
| 0.1 | 0.9724 | 0.9749 | 0.9017 | 0.9100 | 0.8118 | 0.8266 | 0.7217 | 0.7423 | 0.6406 | 0.6658 |
| 0.15 | 0.9714 | 0.9733 | 0.8984 | 0.9049 | 0.8065 | 0.8186 | 0.7151 | 0.7325 | 0.6333 | 0.6554 |
| 0.2 | 0.9700 | 0.9714 | 0.8939 | 0.8990 | 0.7991 | 0.8092 | 0.7060 | 0.7213 | 0.6235 | 0.6437 |

Table 8. T_{NL}/T_L values for amplitude ratio 0.2

| h/a | b/a | Mode number | | | | | | | | | | | | | | | |
|-------|-----|-------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| | | 11 | 12 | 13 | 14 | 21 | 22 | 23 | 24 | 31 | 32 | 33 | 34 | 41 | 42 | 43 | 44 |
| 0.001 | 1 | 0.9801 | 0.9733 | 0.968 | 0.9655 | 0.9733 | 0.9801 | 0.9773 | 0.9733 | 0.968 | 0.9773 | 0.9801 | 0.9786 | 0.9655 | 0.9733 | 0.9786 | 0.9801 |
| | 1.5 | 0.9773 | 0.9786 | 0.9733 | 0.9694 | 0.968 | 0.9773 | 0.9801 | 0.9786 | 0.9647 | 0.9716 | 0.9773 | 0.9799 | 0.9633 | 0.968 | 0.9733 | 0.9773 |
| | 2 | 0.9733 | 0.9801 | 0.9773 | 0.9733 | 0.9655 | 0.9733 | 0.9786 | 0.9801 | 0.9633 | 0.968 | 0.9733 | 0.9773 | 0.9625 | 0.9655 | 0.9694 | 0.9733 |
| | 2.5 | 0.9702 | 0.9792 | 0.9795 | 0.9765 | 0.9641 | 0.9702 | 0.9759 | 0.9792 | 0.9627 | 0.9659 | 0.9702 | 0.9742 | 0.9621 | 0.9641 | 0.967 | 0.9702 |
| | 3 | 0.968 | 0.9773 | 0.9801 | 0.9786 | 0.9633 | 0.968 | 0.9733 | 0.9773 | 0.9623 | 0.9647 | 0.968 | 0.9716 | 0.9619 | 0.9633 | 0.9655 | 0.968 |
| | 5 | 0.9641 | 0.9702 | 0.9759 | 0.9792 | 0.9621 | 0.9641 | 0.967 | 0.9702 | 0.9617 | 0.9627 | 0.9641 | 0.9659 | 0.9616 | 0.9621 | 0.963 | 0.9641 |
| | 10 | 0.9621 | 0.9641 | 0.967 | 0.9702 | 0.9616 | 0.9621 | 0.963 | 0.9641 | 0.9615 | 0.9617 | 0.9621 | 0.9627 | 0.9614 | 0.9616 | 0.9618 | 0.9621 |
| 0.05 | 1 | 0.9799 | 0.9724 | 0.9659 | 0.9616 | 0.9724 | 0.979 | 0.9753 | 0.9697 | 0.9659 | 0.9753 | 0.9777 | 0.975 | 0.9616 | 0.9697 | 0.975 | 0.9758 |
| | 1.5 | 0.9771 | 0.9786 | 0.9733 | 0.9695 | 0.9671 | 0.9766 | 0.9796 | 0.9783 | 0.9625 | 0.9696 | 0.9756 | 0.9786 | 0.9593 | 0.9643 | 0.9699 | 0.9744 |
| | 2 | 0.9731 | 0.9801 | 0.9774 | 0.9736 | 0.9645 | 0.9724 | 0.978 | 0.9798 | 0.9611 | 0.9659 | 0.9714 | 0.9758 | 0.9585 | 0.9616 | 0.9657 | 0.97 |
| | 2.5 | 0.97 | 0.9791 | 0.9795 | 0.9767 | 0.9631 | 0.9693 | 0.9752 | 0.9787 | 0.9604 | 0.9638 | 0.9682 | 0.9724 | 0.9581 | 0.9602 | 0.9632 | 0.9666 |
| | 3 | 0.9678 | 0.9771 | 0.9801 | 0.9788 | 0.9623 | 0.9671 | 0.9725 | 0.9767 | 0.96 | 0.9625 | 0.9659 | 0.9697 | 0.9578 | 0.9593 | 0.9616 | 0.9643 |
| | 5 | 0.9639 | 0.97 | 0.9757 | 0.9791 | 0.9611 | 0.9631 | 0.966 | 0.9693 | 0.9594 | 0.9604 | 0.9619 | 0.9638 | 0.9575 | 0.9581 | 0.959 | 0.9602 |
| | 10 | 0.9619 | 0.9639 | 0.9667 | 0.97 | 0.9606 | 0.9611 | 0.962 | 0.9631 | 0.9592 | 0.9594 | 0.9598 | 0.9604 | 0.9574 | 0.9575 | 0.9577 | 0.9581 |
| 0.1 | 1 | 0.979 | 0.9697 | 0.9595 | 0.9501 | 0.9697 | 0.9758 | 0.9693 | 0.9591 | 0.9595 | 0.9693 | 0.9705 | 0.9644 | 0.9501 | 0.9591 | 0.9644 | 0.9632 |
| | 1.5 | 0.9766 | 0.9783 | 0.9733 | 0.9696 | 0.9643 | 0.9744 | 0.9781 | 0.9773 | 0.9559 | 0.9637 | 0.9707 | 0.9745 | 0.9476 | 0.9533 | 0.9599 | 0.9655 |
| | 2 | 0.9724 | 0.9798 | 0.9777 | 0.9745 | 0.9616 | 0.97 | 0.9762 | 0.9788 | 0.9544 | 0.9597 | 0.9659 | 0.9711 | 0.9467 | 0.9502 | 0.955 | 0.9602 |
| | 2.5 | 0.9693 | 0.9787 | 0.9796 | 0.9774 | 0.9602 | 0.9666 | 0.973 | 0.9772 | 0.9537 | 0.9573 | 0.9622 | 0.9671 | 0.9462 | 0.9486 | 0.952 | 0.9561 |
| | 3 | 0.9671 | 0.9767 | 0.98 | 0.9791 | 0.9593 | 0.9643 | 0.97 | 0.9747 | 0.9533 | 0.9559 | 0.9597 | 0.9639 | 0.946 | 0.9477 | 0.9502 | 0.9534 |
| | 5 | 0.9631 | 0.9693 | 0.9752 | 0.9788 | 0.9581 | 0.9602 | 0.9632 | 0.9666 | 0.9527 | 0.9537 | 0.9553 | 0.9574 | 0.9456 | 0.9462 | 0.9472 | 0.9486 |
| | 10 | 0.9611 | 0.9631 | 0.966 | 0.9693 | 0.9575 | 0.9581 | 0.959 | 0.9602 | 0.9524 | 0.9527 | 0.9531 | 0.9537 | 0.9454 | 0.9456 | 0.9459 | 0.9462 |
| 0.15 | 1 | 0.9777 | 0.9652 | 0.9493 | 0.9319 | 0.9652 | 0.9705 | 0.9597 | 0.9422 | 0.9493 | 0.9597 | 0.9588 | 0.9474 | 0.9319 | 0.9422 | 0.9474 | 0.9432 |
| | 1.5 | 0.9756 | 0.9779 | 0.9731 | 0.9692 | 0.9597 | 0.9707 | 0.9753 | 0.975 | 0.9453 | 0.9542 | 0.9624 | 0.9675 | 0.9291 | 0.9358 | 0.9439 | 0.9511 |
| | 2 | 0.9714 | 0.9794 | 0.9781 | 0.9756 | 0.9568 | 0.9659 | 0.9731 | 0.9768 | 0.9436 | 0.9495 | 0.9567 | 0.9632 | 0.928 | 0.9321 | 0.9379 | 0.9443 |
| | 2.5 | 0.9682 | 0.9781 | 0.9797 | 0.9782 | 0.9553 | 0.9622 | 0.9693 | 0.9744 | 0.9428 | 0.9469 | 0.9525 | 0.9583 | 0.9274 | 0.9302 | 0.9343 | 0.9392 |
| | 3 | 0.9659 | 0.9759 | 0.9798 | 0.9796 | 0.9544 | 0.9597 | 0.966 | 0.9714 | 0.9423 | 0.9453 | 0.9496 | 0.9545 | 0.9271 | 0.9291 | 0.9321 | 0.9359 |
| | 5 | 0.9619 | 0.9682 | 0.9743 | 0.9782 | 0.9531 | 0.9553 | 0.9585 | 0.9622 | 0.9417 | 0.9428 | 0.9446 | 0.9469 | 0.9267 | 0.9274 | 0.9286 | 0.9302 |
| | 10 | 0.9598 | 0.9619 | 0.9648 | 0.9682 | 0.9525 | 0.9531 | 0.954 | 0.9553 | 0.9414 | 0.9417 | 0.9421 | 0.9428 | 0.9265 | 0.9267 | 0.927 | 0.9274 |
| 0.2 | 1 | 0.9758 | 0.9591 | 0.9354 | 0.9082 | 0.9591 | 0.9632 | 0.9466 | 0.9201 | 0.9354 | 0.9466 | 0.9432 | 0.925 | 0.9082 | 0.9201 | 0.925 | 0.9172 |
| | 1.5 | 0.9744 | 0.9773 | 0.9724 | 0.9679 | 0.9533 | 0.9655 | 0.9713 | 0.9713 | 0.931 | 0.9412 | 0.9511 | 0.9576 | 0.9048 | 0.9127 | 0.9227 | 0.9319 |
| | 2 | 0.97 | 0.9788 | 0.9783 | 0.9765 | 0.9502 | 0.9602 | 0.9686 | 0.9735 | 0.9291 | 0.9358 | 0.9443 | 0.9523 | 0.9035 | 0.9084 | 0.9154 | 0.9233 |
| | 2.5 | 0.9666 | 0.9772 | 0.9797 | 0.979 | 0.9486 | 0.9561 | 0.9642 | 0.9704 | 0.9282 | 0.9328 | 0.9392 | 0.9462 | 0.9029 | 0.9061 | 0.911 | 0.917 |



| | | | | | | | | | | | | | | | | |
|----|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| 3 | 0.9643 | 0.9747 | 0.9793 | 0.98 | 0.9477 | 0.9534 | 0.9603 | 0.9666 | 0.9277 | 0.931 | 0.9359 | 0.9416 | 0.9026 | 0.9048 | 0.9084 | 0.913 |
| 5 | 0.9602 | 0.9666 | 0.9731 | 0.9774 | 0.9462 | 0.9486 | 0.9521 | 0.9562 | 0.9269 | 0.9282 | 0.9302 | 0.9328 | 0.9021 | 0.9029 | 0.9043 | 0.9061 |
| 10 | 0.9581 | 0.9602 | 0.9632 | 0.9666 | 0.9456 | 0.9462 | 0.9472 | 0.9486 | 0.9266 | 0.9269 | 0.9274 | 0.9282 | 0.9018 | 0.9021 | 0.9024 | 0.9029 |

Table 9. T_{NL}/T_L values for amplitude ratio 0.4

| h/a | b/a | Mode number | | | | | | | | | | | | | | | |
|-------|-----|-------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| | | 11 | 12 | 13 | 14 | 21 | 22 | 23 | 24 | 31 | 32 | 33 | 34 | 41 | 42 | 43 | 44 |
| 0.001 | 1 | 0.927 | 0.9043 | 0.8878 | 0.88 | 0.9043 | 0.927 | 0.9175 | 0.9043 | 0.8878 | 0.9175 | 0.927 | 0.9219 | 0.88 | 0.9043 | 0.9219 | 0.927 |
| | 1.5 | 0.9175 | 0.9219 | 0.9043 | 0.892 | 0.8878 | 0.9175 | 0.927 | 0.9219 | 0.8777 | 0.899 | 0.9175 | 0.9261 | 0.8736 | 0.8878 | 0.9043 | 0.9175 |
| | 2 | 0.9043 | 0.927 | 0.9175 | 0.9043 | 0.88 | 0.9043 | 0.9219 | 0.927 | 0.8736 | 0.8878 | 0.9043 | 0.9175 | 0.8712 | 0.88 | 0.892 | 0.9043 |
| | 2.5 | 0.8945 | 0.9238 | 0.9248 | 0.9147 | 0.8759 | 0.8945 | 0.9128 | 0.9238 | 0.8716 | 0.8815 | 0.8945 | 0.9074 | 0.87 | 0.8759 | 0.8846 | 0.8945 |
| | 3 | 0.8878 | 0.9175 | 0.927 | 0.9219 | 0.8736 | 0.8878 | 0.9043 | 0.9175 | 0.8705 | 0.8777 | 0.8878 | 0.899 | 0.8694 | 0.8736 | 0.88 | 0.8878 |
| | 5 | 0.8759 | 0.8945 | 0.9128 | 0.9238 | 0.87 | 0.8759 | 0.8846 | 0.8945 | 0.8688 | 0.8716 | 0.8759 | 0.8815 | 0.8684 | 0.87 | 0.8725 | 0.8759 |
| | 10 | 0.87 | 0.8759 | 0.8846 | 0.8945 | 0.8684 | 0.87 | 0.8725 | 0.8759 | 0.8681 | 0.8688 | 0.87 | 0.8716 | 0.868 | 0.8684 | 0.8691 | 0.87 |
| 0.05 | 1 | 0.926 | 0.9014 | 0.8813 | 0.8683 | 0.9014 | 0.9233 | 0.9109 | 0.8929 | 0.8813 | 0.9109 | 0.9188 | 0.9099 | 0.8683 | 0.8929 | 0.9099 | 0.9126 |
| | 1.5 | 0.9169 | 0.9217 | 0.9044 | 0.8924 | 0.8849 | 0.915 | 0.9253 | 0.921 | 0.871 | 0.8928 | 0.912 | 0.9217 | 0.8618 | 0.8764 | 0.8936 | 0.9079 |
| | 2 | 0.9037 | 0.9267 | 0.9179 | 0.9055 | 0.8771 | 0.9017 | 0.9199 | 0.926 | 0.8669 | 0.8814 | 0.8984 | 0.9124 | 0.8594 | 0.8684 | 0.8808 | 0.8939 |
| | 2.5 | 0.8938 | 0.9234 | 0.925 | 0.9155 | 0.873 | 0.8917 | 0.9105 | 0.9222 | 0.8649 | 0.8749 | 0.8883 | 0.9017 | 0.8582 | 0.8642 | 0.8731 | 0.8835 |
| | 3 | 0.8871 | 0.9169 | 0.9269 | 0.9223 | 0.8706 | 0.885 | 0.9017 | 0.9154 | 0.8638 | 0.8711 | 0.8814 | 0.8929 | 0.8576 | 0.8618 | 0.8684 | 0.8765 |
| | 5 | 0.8752 | 0.8938 | 0.9123 | 0.9235 | 0.867 | 0.873 | 0.8817 | 0.8918 | 0.8621 | 0.8649 | 0.8693 | 0.8749 | 0.8566 | 0.8582 | 0.8608 | 0.8642 |
| | 10 | 0.8692 | 0.8752 | 0.8839 | 0.8938 | 0.8654 | 0.867 | 0.8696 | 0.873 | 0.8614 | 0.8621 | 0.8633 | 0.8649 | 0.8562 | 0.8566 | 0.8573 | 0.8582 |
| 0.1 | 1 | 0.9233 | 0.8929 | 0.8625 | 0.836 | 0.8929 | 0.9126 | 0.8919 | 0.8612 | 0.8625 | 0.8919 | 0.8955 | 0.8767 | 0.836 | 0.8612 | 0.8767 | 0.8732 |
| | 1.5 | 0.915 | 0.921 | 0.9044 | 0.8927 | 0.8764 | 0.9079 | 0.9201 | 0.9173 | 0.852 | 0.8748 | 0.8961 | 0.9083 | 0.8293 | 0.8447 | 0.8635 | 0.8803 |
| | 2 | 0.9017 | 0.926 | 0.9189 | 0.9084 | 0.8684 | 0.8939 | 0.9139 | 0.9226 | 0.8478 | 0.8629 | 0.8812 | 0.8974 | 0.8267 | 0.8362 | 0.8495 | 0.8642 |
| | 2.5 | 0.8917 | 0.9222 | 0.9253 | 0.9177 | 0.8642 | 0.8835 | 0.9034 | 0.9171 | 0.8457 | 0.8561 | 0.8703 | 0.885 | 0.8255 | 0.8318 | 0.8412 | 0.8526 |
| | 3 | 0.885 | 0.9154 | 0.9265 | 0.9236 | 0.8618 | 0.8765 | 0.894 | 0.909 | 0.8446 | 0.8521 | 0.8629 | 0.8753 | 0.8248 | 0.8293 | 0.8362 | 0.8449 |
| | 5 | 0.873 | 0.8918 | 0.9106 | 0.9224 | 0.8582 | 0.8642 | 0.8732 | 0.8836 | 0.8429 | 0.8458 | 0.8503 | 0.8561 | 0.8238 | 0.8255 | 0.8282 | 0.8318 |
| | 10 | 0.867 | 0.873 | 0.8817 | 0.8918 | 0.8566 | 0.8582 | 0.8608 | 0.8642 | 0.8422 | 0.8429 | 0.8441 | 0.8458 | 0.8234 | 0.8238 | 0.8245 | 0.8255 |
| 0.15 | 1 | 0.9188 | 0.8793 | 0.8336 | 0.7892 | 0.8793 | 0.8955 | 0.8629 | 0.815 | 0.8336 | 0.8629 | 0.8604 | 0.8285 | 0.7892 | 0.815 | 0.8285 | 0.8175 |
| | 1.5 | 0.912 | 0.9196 | 0.9037 | 0.8914 | 0.8628 | 0.8961 | 0.9109 | 0.9098 | 0.823 | 0.8471 | 0.871 | 0.8861 | 0.7823 | 0.7986 | 0.8193 | 0.8387 |
| | 2 | 0.8984 | 0.9246 | 0.92 | 0.9119 | 0.8545 | 0.8812 | 0.9038 | 0.9158 | 0.8186 | 0.8344 | 0.8544 | 0.8733 | 0.7797 | 0.7896 | 0.8039 | 0.8204 |
| | 2.5 | 0.8883 | 0.9201 | 0.9256 | 0.9206 | 0.8503 | 0.8703 | 0.8918 | 0.9081 | 0.8165 | 0.8272 | 0.8424 | 0.8588 | 0.7784 | 0.785 | 0.795 | 0.8073 |
| | 3 | 0.8814 | 0.9127 | 0.9257 | 0.9251 | 0.8478 | 0.8629 | 0.8815 | 0.8983 | 0.8153 | 0.823 | 0.8345 | 0.848 | 0.7777 | 0.7823 | 0.7896 | 0.7989 |
| | 5 | 0.8693 | 0.8883 | 0.9077 | 0.9206 | 0.8441 | 0.8503 | 0.8595 | 0.8704 | 0.8135 | 0.8165 | 0.8211 | 0.8273 | 0.7767 | 0.7784 | 0.7812 | 0.785 |
| | 10 | 0.8633 | 0.8693 | 0.8781 | 0.8883 | 0.8425 | 0.8441 | 0.8467 | 0.8503 | 0.8128 | 0.8135 | 0.8148 | 0.8165 | 0.7763 | 0.7767 | 0.7774 | 0.7784 |
| 0.2 | 1 | 0.9126 | 0.8612 | 0.7977 | 0.7352 | 0.8612 | 0.8732 | 0.8265 | 0.7613 | 0.7977 | 0.8265 | 0.8175 | 0.7727 | 0.7352 | 0.7613 | 0.7727 | 0.7548 |
| | 1.5 | 0.9079 | 0.9173 | 0.9016 | 0.8875 | 0.8447 | 0.8803 | 0.8979 | 0.898 | 0.7868 | 0.8123 | 0.8387 | 0.8569 | 0.7282 | 0.7451 | 0.7672 | 0.789 |
| | 2 | 0.8939 | 0.9226 | 0.9208 | 0.9148 | 0.8362 | 0.8642 | 0.8897 | 0.9052 | 0.7823 | 0.7988 | 0.8204 | 0.8419 | 0.7255 | 0.7357 | 0.7508 | 0.7687 |
| | 2.5 | 0.8835 | 0.9171 | 0.9254 | 0.9233 | 0.8318 | 0.8526 | 0.8761 | 0.8952 | 0.7801 | 0.7913 | 0.8073 | 0.8254 | 0.7242 | 0.7309 | 0.7414 | 0.7545 |



| | | | | | | | | | | | | | | | | |
|----|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| 3 | 0.8765 | 0.909 | 0.9243 | 0.9264 | 0.8293 | 0.8449 | 0.8647 | 0.8836 | 0.7789 | 0.7869 | 0.7989 | 0.8134 | 0.7235 | 0.7283 | 0.7358 | 0.7455 |
| 5 | 0.8642 | 0.8836 | 0.9037 | 0.9178 | 0.8255 | 0.8318 | 0.8413 | 0.8528 | 0.7771 | 0.7801 | 0.785 | 0.7914 | 0.7225 | 0.7242 | 0.7271 | 0.731 |
| 10 | 0.8582 | 0.8642 | 0.8732 | 0.8836 | 0.8238 | 0.8255 | 0.8282 | 0.8318 | 0.7764 | 0.7771 | 0.7784 | 0.7801 | 0.7221 | 0.7225 | 0.7232 | 0.7242 |

Table 10. T_{NL}/T_L values for amplitude ratio 0.6

| h/a | b/a | Mode number | | | | | | | | | | | | | | | |
|-------|-----|-------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| | | 11 | 12 | 13 | 14 | 21 | 22 | 23 | 24 | 31 | 32 | 33 | 34 | 41 | 42 | 43 | 44 |
| 0.001 | 1 | 0.8548 | 0.8162 | 0.7895 | 0.7772 | 0.8162 | 0.8548 | 0.8383 | 0.8162 | 0.7895 | 0.8383 | 0.8548 | 0.8459 | 0.7772 | 0.8162 | 0.8459 | 0.8548 |
| | 1.5 | 0.8383 | 0.8459 | 0.8162 | 0.7961 | 0.7895 | 0.8383 | 0.8548 | 0.8459 | 0.7736 | 0.8074 | 0.8383 | 0.8532 | 0.7673 | 0.7895 | 0.8162 | 0.8383 |
| | 2 | 0.8162 | 0.8548 | 0.8383 | 0.8162 | 0.7772 | 0.8162 | 0.8459 | 0.8548 | 0.7673 | 0.7895 | 0.8162 | 0.8383 | 0.7636 | 0.7772 | 0.7961 | 0.8162 |
| | 2.5 | 0.8002 | 0.8493 | 0.8511 | 0.8335 | 0.7709 | 0.8002 | 0.8304 | 0.8493 | 0.7643 | 0.7795 | 0.8002 | 0.8212 | 0.7619 | 0.7709 | 0.7844 | 0.8002 |
| | 3 | 0.7895 | 0.8383 | 0.8548 | 0.8459 | 0.7673 | 0.7895 | 0.8162 | 0.8383 | 0.7626 | 0.7736 | 0.7895 | 0.8074 | 0.7609 | 0.7673 | 0.7772 | 0.7895 |
| | 5 | 0.7709 | 0.8002 | 0.8304 | 0.8493 | 0.7619 | 0.7709 | 0.7844 | 0.8002 | 0.7601 | 0.7643 | 0.7709 | 0.7795 | 0.7595 | 0.7619 | 0.7657 | 0.7709 |
| | 10 | 0.7619 | 0.7709 | 0.7844 | 0.8002 | 0.7595 | 0.7619 | 0.7658 | 0.7709 | 0.7591 | 0.7601 | 0.7619 | 0.7643 | 0.7589 | 0.7595 | 0.7605 | 0.7619 |
| 0.05 | 1 | 0.8532 | 0.8114 | 0.7792 | 0.7594 | 0.8114 | 0.8484 | 0.8271 | 0.7976 | 0.7792 | 0.8271 | 0.8406 | 0.8255 | 0.7594 | 0.7976 | 0.8255 | 0.83 |
| | 1.5 | 0.8372 | 0.8455 | 0.8163 | 0.7967 | 0.7849 | 0.8342 | 0.8519 | 0.8443 | 0.7635 | 0.7973 | 0.8291 | 0.8456 | 0.7497 | 0.7717 | 0.7987 | 0.822 |
| | 2 | 0.8151 | 0.8544 | 0.839 | 0.8181 | 0.7727 | 0.8118 | 0.8425 | 0.8531 | 0.7573 | 0.7794 | 0.8065 | 0.8297 | 0.7461 | 0.7595 | 0.7785 | 0.7991 |
| | 2.5 | 0.7991 | 0.8486 | 0.8513 | 0.8349 | 0.7664 | 0.7957 | 0.8264 | 0.8465 | 0.7543 | 0.7694 | 0.7902 | 0.8118 | 0.7443 | 0.7533 | 0.7667 | 0.7827 |
| | 3 | 0.7883 | 0.8374 | 0.8546 | 0.8467 | 0.7628 | 0.785 | 0.8119 | 0.8347 | 0.7526 | 0.7635 | 0.7794 | 0.7976 | 0.7434 | 0.7497 | 0.7595 | 0.7718 |
| | 5 | 0.7698 | 0.7991 | 0.8294 | 0.8487 | 0.7574 | 0.7664 | 0.7798 | 0.7957 | 0.7501 | 0.7543 | 0.7608 | 0.7694 | 0.742 | 0.7443 | 0.7481 | 0.7533 |
| | 10 | 0.7608 | 0.7698 | 0.7832 | 0.7991 | 0.755 | 0.7574 | 0.7612 | 0.7664 | 0.7491 | 0.7501 | 0.7519 | 0.7543 | 0.7414 | 0.742 | 0.743 | 0.7443 |
| 0.1 | 1 | 0.8484 | 0.7976 | 0.7506 | 0.7125 | 0.7976 | 0.83 | 0.796 | 0.7487 | 0.7506 | 0.796 | 0.8018 | 0.7721 | 0.7125 | 0.7487 | 0.7721 | 0.7667 |
| | 1.5 | 0.8342 | 0.8443 | 0.8164 | 0.7972 | 0.7717 | 0.822 | 0.8428 | 0.8381 | 0.7354 | 0.7692 | 0.8027 | 0.8228 | 0.7033 | 0.7248 | 0.7522 | 0.7776 |
| | 2 | 0.8118 | 0.8531 | 0.8407 | 0.8229 | 0.7595 | 0.7991 | 0.8323 | 0.8471 | 0.7293 | 0.7512 | 0.7791 | 0.8049 | 0.6998 | 0.7128 | 0.7318 | 0.7533 |
| | 2.5 | 0.7957 | 0.8465 | 0.8519 | 0.8387 | 0.7533 | 0.7827 | 0.8147 | 0.8377 | 0.7263 | 0.7413 | 0.7623 | 0.785 | 0.6981 | 0.7067 | 0.7199 | 0.7362 |
| | 3 | 0.785 | 0.8347 | 0.854 | 0.8488 | 0.7497 | 0.7718 | 0.7994 | 0.8239 | 0.7247 | 0.7354 | 0.7513 | 0.77 | 0.6972 | 0.7033 | 0.7129 | 0.7251 |
| | 5 | 0.7664 | 0.7957 | 0.8266 | 0.8468 | 0.7443 | 0.7533 | 0.7667 | 0.7828 | 0.7223 | 0.7263 | 0.7328 | 0.7413 | 0.6959 | 0.6981 | 0.7018 | 0.7067 |
| | 10 | 0.7574 | 0.7664 | 0.7798 | 0.7957 | 0.742 | 0.7443 | 0.7481 | 0.7533 | 0.7213 | 0.7223 | 0.724 | 0.7263 | 0.6953 | 0.6959 | 0.6968 | 0.6981 |
| 0.15 | 1 | 0.8406 | 0.7761 | 0.7093 | 0.6506 | 0.7761 | 0.8018 | 0.7512 | 0.684 | 0.7093 | 0.7512 | 0.7475 | 0.7022 | 0.6506 | 0.684 | 0.7022 | 0.6873 |
| | 1.5 | 0.8291 | 0.8419 | 0.8151 | 0.7951 | 0.7511 | 0.8027 | 0.8272 | 0.8253 | 0.6947 | 0.7282 | 0.7634 | 0.7868 | 0.642 | 0.6625 | 0.6897 | 0.7164 |
| | 2 | 0.8065 | 0.8507 | 0.8427 | 0.8287 | 0.739 | 0.7791 | 0.8153 | 0.8354 | 0.6888 | 0.7103 | 0.7387 | 0.7669 | 0.6387 | 0.6511 | 0.6695 | 0.6912 |
| | 2.5 | 0.7902 | 0.8429 | 0.8524 | 0.8436 | 0.7328 | 0.7623 | 0.7959 | 0.8225 | 0.6859 | 0.7005 | 0.7215 | 0.7453 | 0.6371 | 0.6453 | 0.6579 | 0.6739 |
| | 3 | 0.7794 | 0.8302 | 0.8525 | 0.8515 | 0.7293 | 0.7513 | 0.7796 | 0.8063 | 0.6844 | 0.6948 | 0.7105 | 0.7295 | 0.6363 | 0.642 | 0.6511 | 0.663 |
| | 5 | 0.7608 | 0.7903 | 0.8218 | 0.8436 | 0.724 | 0.7328 | 0.7462 | 0.7625 | 0.6821 | 0.6859 | 0.6922 | 0.7006 | 0.635 | 0.6371 | 0.6406 | 0.6453 |
| | 10 | 0.7519 | 0.7608 | 0.7743 | 0.7903 | 0.7217 | 0.724 | 0.7277 | 0.7328 | 0.6811 | 0.6821 | 0.6837 | 0.686 | 0.6345 | 0.635 | 0.6359 | 0.6371 |
| 0.2 | 1 | 0.83 | 0.7487 | 0.6614 | 0.586 | 0.7487 | 0.7667 | 0.6996 | 0.6164 | 0.6614 | 0.6996 | 0.6873 | 0.6302 | 0.586 | 0.6164 | 0.6302 | 0.6087 |
| | 1.5 | 0.822 | 0.8381 | 0.8117 | 0.7889 | 0.7248 | 0.7776 | 0.8056 | 0.8058 | 0.6476 | 0.6804 | 0.7164 | 0.7424 | 0.5781 | 0.5973 | 0.6235 | 0.6503 |
| | 2 | 0.7991 | 0.8471 | 0.8441 | 0.8337 | 0.7128 | 0.7533 | 0.7925 | 0.8176 | 0.642 | 0.6628 | 0.6912 | 0.7209 | 0.575 | 0.5865 | 0.604 | 0.6252 |
| | 2.5 | 0.7827 | 0.8377 | 0.8521 | 0.8483 | 0.7067 | 0.7362 | 0.7711 | 0.8013 | 0.6393 | 0.6533 | 0.6739 | 0.698 | 0.5736 | 0.5811 | 0.593 | 0.6083 |



| | | | | | | | | | | | | | | | | |
|----|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| 3 | 0.7718 | 0.8239 | 0.8501 | 0.8538 | 0.7033 | 0.7251 | 0.754 | 0.7828 | 0.6378 | 0.6477 | 0.663 | 0.6819 | 0.5728 | 0.5781 | 0.5866 | 0.5978 |
| 5 | 0.7533 | 0.7828 | 0.8151 | 0.8389 | 0.6981 | 0.7067 | 0.72 | 0.7364 | 0.6356 | 0.6393 | 0.6453 | 0.6533 | 0.5716 | 0.5736 | 0.5768 | 0.5811 |
| 10 | 0.7443 | 0.7533 | 0.7667 | 0.7828 | 0.6959 | 0.6981 | 0.7018 | 0.7068 | 0.6346 | 0.6356 | 0.6371 | 0.6393 | 0.5711 | 0.5716 | 0.5724 | 0.5736 |

Table 11. T_{NL}/T_L values for amplitude ratio 0.8

| h/a | b/a | Mode number | | | | | | | | | | | | | | | |
|-------|-----|-------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| | | 11 | 12 | 13 | 14 | 21 | 22 | 23 | 24 | 31 | 32 | 33 | 34 | 41 | 42 | 43 | 44 |
| 0.001 | 1 | 0.7773 | 0.7272 | 0.6943 | 0.6796 | 0.7272 | 0.7773 | 0.7555 | 0.7272 | 0.6943 | 0.7555 | 0.7773 | 0.7655 | 0.6796 | 0.7272 | 0.7655 | 0.7773 |
| | 1.5 | 0.7555 | 0.7655 | 0.7272 | 0.7024 | 0.6943 | 0.7555 | 0.7773 | 0.7655 | 0.6753 | 0.7162 | 0.7555 | 0.7752 | 0.6679 | 0.6943 | 0.7272 | 0.7555 |
| | 2 | 0.7272 | 0.7773 | 0.7555 | 0.7272 | 0.6796 | 0.7272 | 0.7655 | 0.7773 | 0.6679 | 0.6943 | 0.7272 | 0.7555 | 0.6636 | 0.6796 | 0.7024 | 0.7272 |
| | 2.5 | 0.7073 | 0.77 | 0.7723 | 0.7493 | 0.6721 | 0.7073 | 0.7453 | 0.77 | 0.6644 | 0.6823 | 0.7073 | 0.7335 | 0.6616 | 0.6721 | 0.6881 | 0.7073 |
| | 3 | 0.6943 | 0.7555 | 0.7773 | 0.7655 | 0.6679 | 0.6943 | 0.7272 | 0.7555 | 0.6624 | 0.6753 | 0.6943 | 0.7162 | 0.6605 | 0.6679 | 0.6796 | 0.6943 |
| | 5 | 0.6721 | 0.7073 | 0.7453 | 0.77 | 0.6616 | 0.6721 | 0.6881 | 0.7073 | 0.6595 | 0.6644 | 0.6721 | 0.6823 | 0.6588 | 0.6616 | 0.6661 | 0.6721 |
| | 10 | 0.6616 | 0.6721 | 0.6881 | 0.7073 | 0.6588 | 0.6616 | 0.6661 | 0.6721 | 0.6583 | 0.6595 | 0.6616 | 0.6644 | 0.6581 | 0.6588 | 0.66 | 0.6616 |
| 0.05 | 1 | 0.7751 | 0.7212 | 0.682 | 0.6587 | 0.7212 | 0.7687 | 0.741 | 0.7042 | 0.682 | 0.741 | 0.7585 | 0.739 | 0.6587 | 0.7042 | 0.739 | 0.7447 |
| | 1.5 | 0.7541 | 0.765 | 0.7274 | 0.7031 | 0.6888 | 0.7501 | 0.7734 | 0.7633 | 0.6634 | 0.7038 | 0.7436 | 0.7651 | 0.6475 | 0.6731 | 0.7056 | 0.7346 |
| | 2 | 0.7258 | 0.7767 | 0.7564 | 0.7296 | 0.6742 | 0.7217 | 0.761 | 0.775 | 0.6562 | 0.6822 | 0.7151 | 0.7444 | 0.6433 | 0.6588 | 0.6811 | 0.706 |
| | 2.5 | 0.7059 | 0.769 | 0.7727 | 0.7512 | 0.6668 | 0.7018 | 0.7402 | 0.7662 | 0.6527 | 0.6704 | 0.6952 | 0.7217 | 0.6414 | 0.6516 | 0.6671 | 0.6861 |
| | 3 | 0.6929 | 0.7543 | 0.777 | 0.7665 | 0.6626 | 0.6888 | 0.7218 | 0.7508 | 0.6508 | 0.6634 | 0.6822 | 0.7041 | 0.6403 | 0.6475 | 0.6588 | 0.6732 |
| | 5 | 0.6708 | 0.706 | 0.7441 | 0.7692 | 0.6563 | 0.6668 | 0.6827 | 0.7019 | 0.648 | 0.6527 | 0.6603 | 0.6704 | 0.6387 | 0.6414 | 0.6457 | 0.6516 |
| | 10 | 0.6603 | 0.6708 | 0.6868 | 0.706 | 0.6536 | 0.6563 | 0.6608 | 0.6668 | 0.6468 | 0.648 | 0.65 | 0.6527 | 0.638 | 0.6387 | 0.6398 | 0.6414 |
| 0.1 | 1 | 0.7687 | 0.7042 | 0.6485 | 0.6059 | 0.7042 | 0.7447 | 0.7022 | 0.6464 | 0.6485 | 0.7022 | 0.7093 | 0.6735 | 0.6059 | 0.6464 | 0.6735 | 0.6672 |
| | 1.5 | 0.7501 | 0.7633 | 0.7274 | 0.7037 | 0.6731 | 0.7346 | 0.7614 | 0.7552 | 0.6312 | 0.6701 | 0.7105 | 0.7356 | 0.5958 | 0.6194 | 0.6504 | 0.6801 |
| | 2 | 0.7217 | 0.775 | 0.7587 | 0.7357 | 0.6588 | 0.706 | 0.7477 | 0.767 | 0.6244 | 0.6492 | 0.6818 | 0.7131 | 0.5921 | 0.6062 | 0.6272 | 0.6516 |
| | 2.5 | 0.7018 | 0.7662 | 0.7734 | 0.756 | 0.6516 | 0.6861 | 0.7253 | 0.7547 | 0.6211 | 0.6379 | 0.662 | 0.6889 | 0.5903 | 0.5996 | 0.614 | 0.6321 |
| | 3 | 0.6888 | 0.7508 | 0.7761 | 0.7693 | 0.6475 | 0.6732 | 0.7063 | 0.737 | 0.6193 | 0.6313 | 0.6493 | 0.6711 | 0.5893 | 0.5959 | 0.6063 | 0.6198 |
| | 5 | 0.6668 | 0.7019 | 0.7404 | 0.7667 | 0.6414 | 0.6516 | 0.6672 | 0.6862 | 0.6166 | 0.6211 | 0.6283 | 0.6379 | 0.5879 | 0.5903 | 0.5942 | 0.5996 |
| | 10 | 0.6563 | 0.6668 | 0.6827 | 0.7019 | 0.6387 | 0.6414 | 0.6457 | 0.6516 | 0.6155 | 0.6166 | 0.6185 | 0.6211 | 0.5873 | 0.5879 | 0.5889 | 0.5903 |
| 0.15 | 1 | 0.7585 | 0.6783 | 0.6023 | 0.5406 | 0.6783 | 0.7093 | 0.6492 | 0.5753 | 0.6023 | 0.6492 | 0.645 | 0.5947 | 0.5406 | 0.5753 | 0.5947 | 0.5788 |
| | 1.5 | 0.7436 | 0.7602 | 0.7259 | 0.7011 | 0.6491 | 0.7105 | 0.7412 | 0.7387 | 0.5866 | 0.6232 | 0.6633 | 0.6911 | 0.5318 | 0.5528 | 0.5813 | 0.6101 |
| | 2 | 0.7151 | 0.7718 | 0.7613 | 0.7431 | 0.6353 | 0.6818 | 0.7261 | 0.7517 | 0.5803 | 0.6035 | 0.635 | 0.6674 | 0.5285 | 0.541 | 0.56 | 0.5829 |
| | 2.5 | 0.6952 | 0.7614 | 0.774 | 0.7625 | 0.6283 | 0.662 | 0.702 | 0.7352 | 0.5773 | 0.5929 | 0.6158 | 0.6424 | 0.5269 | 0.5351 | 0.5481 | 0.5646 |
| | 3 | 0.6822 | 0.745 | 0.7743 | 0.7729 | 0.6244 | 0.6493 | 0.6824 | 0.7149 | 0.5756 | 0.5867 | 0.6037 | 0.6246 | 0.5261 | 0.5318 | 0.5411 | 0.5533 |
| | 5 | 0.6603 | 0.6952 | 0.7343 | 0.7625 | 0.6185 | 0.6283 | 0.6435 | 0.6622 | 0.5732 | 0.5773 | 0.584 | 0.5929 | 0.5248 | 0.5269 | 0.5304 | 0.5352 |
| | 10 | 0.65 | 0.6603 | 0.6761 | 0.6953 | 0.616 | 0.6185 | 0.6227 | 0.6283 | 0.5721 | 0.5732 | 0.5749 | 0.5773 | 0.5243 | 0.5248 | 0.5257 | 0.5269 |
| 0.2 | 1 | 0.7447 | 0.6464 | 0.5516 | 0.4767 | 0.6464 | 0.6672 | 0.5918 | 0.5063 | 0.5516 | 0.5918 | 0.5788 | 0.5199 | 0.4767 | 0.5063 | 0.5199 | 0.4987 |
| | 1.5 | 0.7346 | 0.7552 | 0.7216 | 0.6936 | 0.6194 | 0.6801 | 0.7141 | 0.7143 | 0.5376 | 0.5714 | 0.6101 | 0.6391 | 0.4692 | 0.4876 | 0.5133 | 0.5403 |
| | 2 | 0.706 | 0.767 | 0.7631 | 0.7495 | 0.6062 | 0.6516 | 0.6979 | 0.729 | 0.5318 | 0.5531 | 0.5829 | 0.6151 | 0.4663 | 0.4773 | 0.4941 | 0.515 |
| | 2.5 | 0.6861 | 0.7547 | 0.7736 | 0.7687 | 0.5996 | 0.6321 | 0.6724 | 0.7087 | 0.5291 | 0.5433 | 0.5646 | 0.5902 | 0.4649 | 0.4721 | 0.4835 | 0.4983 |



| | | | | | | | | | | | | | | | | | |
|--|----|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| | 3 | 0.6732 | 0.737 | 0.771 | 0.776 | 0.5959 | 0.6198 | 0.6524 | 0.6862 | 0.5276 | 0.5377 | 0.5533 | 0.573 | 0.4642 | 0.4692 | 0.4773 | 0.4882 |
| | 5 | 0.6516 | 0.6862 | 0.7259 | 0.7563 | 0.5903 | 0.5996 | 0.6141 | 0.6324 | 0.5254 | 0.5291 | 0.5352 | 0.5434 | 0.4631 | 0.4649 | 0.468 | 0.4721 |
| | 10 | 0.6414 | 0.6516 | 0.6672 | 0.6863 | 0.5879 | 0.5903 | 0.5942 | 0.5996 | 0.5244 | 0.5254 | 0.5269 | 0.5291 | 0.4626 | 0.4631 | 0.4639 | 0.4649 |

Table 12. T_{NL}/T_L values for amplitude ratio 1.0

| h/a | b/a | Mode number | | | | | | | | | | | | | | | |
|-------|-----|-------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| | | 11 | 12 | 13 | 14 | 21 | 22 | 23 | 24 | 31 | 32 | 33 | 34 | 41 | 42 | 43 | 44 |
| 0.001 | 1 | 0.7029 | 0.6465 | 0.611 | 0.5954 | 0.6465 | 0.7029 | 0.678 | 0.6465 | 0.611 | 0.678 | 0.7029 | 0.6894 | 0.5954 | 0.6465 | 0.6894 | 0.7029 |
| | 1.5 | 0.678 | 0.6894 | 0.6465 | 0.6196 | 0.611 | 0.678 | 0.7029 | 0.6894 | 0.5909 | 0.6346 | 0.678 | 0.7005 | 0.5832 | 0.611 | 0.6465 | 0.678 |
| | 2 | 0.6465 | 0.7029 | 0.678 | 0.6465 | 0.5954 | 0.6465 | 0.6894 | 0.7029 | 0.5832 | 0.611 | 0.6465 | 0.678 | 0.5787 | 0.5954 | 0.6196 | 0.6465 |
| | 2.5 | 0.6249 | 0.6945 | 0.6972 | 0.6711 | 0.5876 | 0.6249 | 0.6666 | 0.6945 | 0.5795 | 0.5983 | 0.6249 | 0.6535 | 0.5766 | 0.5876 | 0.6044 | 0.6249 |
| | 3 | 0.611 | 0.678 | 0.7029 | 0.6894 | 0.5832 | 0.611 | 0.6465 | 0.678 | 0.5775 | 0.5909 | 0.611 | 0.6346 | 0.5755 | 0.5832 | 0.5954 | 0.611 |
| | 5 | 0.5876 | 0.6249 | 0.6666 | 0.6945 | 0.5766 | 0.5876 | 0.6044 | 0.6249 | 0.5745 | 0.5795 | 0.5876 | 0.5983 | 0.5738 | 0.5766 | 0.5813 | 0.5876 |
| | 10 | 0.5766 | 0.5876 | 0.6044 | 0.6249 | 0.5738 | 0.5766 | 0.5813 | 0.5876 | 0.5733 | 0.5745 | 0.5766 | 0.5795 | 0.5731 | 0.5738 | 0.575 | 0.5766 |
| 0.05 | 1 | 0.7005 | 0.64 | 0.5979 | 0.5736 | 0.64 | 0.6931 | 0.6618 | 0.6215 | 0.5979 | 0.6618 | 0.6814 | 0.6596 | 0.5736 | 0.6215 | 0.6596 | 0.666 |
| | 1.5 | 0.6765 | 0.6888 | 0.6468 | 0.6204 | 0.6051 | 0.672 | 0.6984 | 0.687 | 0.5785 | 0.6212 | 0.6647 | 0.689 | 0.5621 | 0.5886 | 0.623 | 0.6547 |
| | 2 | 0.645 | 0.7023 | 0.6791 | 0.6492 | 0.5898 | 0.6406 | 0.6843 | 0.7003 | 0.5711 | 0.5981 | 0.6333 | 0.6656 | 0.5579 | 0.5737 | 0.597 | 0.6235 |
| | 2.5 | 0.6234 | 0.6935 | 0.6976 | 0.6732 | 0.5821 | 0.619 | 0.6609 | 0.6902 | 0.5675 | 0.5857 | 0.6119 | 0.6405 | 0.5559 | 0.5663 | 0.5824 | 0.6023 |
| | 3 | 0.6095 | 0.6767 | 0.7027 | 0.6906 | 0.5777 | 0.6052 | 0.6407 | 0.6728 | 0.5656 | 0.5786 | 0.5982 | 0.6215 | 0.5548 | 0.5622 | 0.5738 | 0.5887 |
| | 5 | 0.5862 | 0.6235 | 0.6652 | 0.6936 | 0.5712 | 0.5821 | 0.5987 | 0.6191 | 0.5627 | 0.5675 | 0.5754 | 0.5858 | 0.5532 | 0.5559 | 0.5603 | 0.5663 |
| | 10 | 0.5753 | 0.5862 | 0.603 | 0.6235 | 0.5684 | 0.5712 | 0.5758 | 0.5821 | 0.5614 | 0.5627 | 0.5647 | 0.5675 | 0.5525 | 0.5532 | 0.5543 | 0.5559 |
| 0.1 | 1 | 0.6931 | 0.6215 | 0.5632 | 0.5203 | 0.6215 | 0.666 | 0.6195 | 0.561 | 0.5632 | 0.6195 | 0.627 | 0.589 | 0.5203 | 0.561 | 0.589 | 0.5825 |
| | 1.5 | 0.672 | 0.687 | 0.6468 | 0.621 | 0.5886 | 0.6547 | 0.6847 | 0.6777 | 0.5456 | 0.5855 | 0.6283 | 0.6558 | 0.5104 | 0.5337 | 0.5651 | 0.5959 |
| | 2 | 0.6406 | 0.7003 | 0.6816 | 0.6559 | 0.5737 | 0.6235 | 0.6693 | 0.6911 | 0.5388 | 0.5639 | 0.5978 | 0.6312 | 0.5067 | 0.5206 | 0.5415 | 0.5664 |
| | 2.5 | 0.619 | 0.6902 | 0.6985 | 0.6787 | 0.5663 | 0.6023 | 0.6445 | 0.6771 | 0.5355 | 0.5524 | 0.5771 | 0.6053 | 0.5049 | 0.5141 | 0.5284 | 0.5465 |
| | 3 | 0.6052 | 0.6728 | 0.7016 | 0.6938 | 0.5622 | 0.5887 | 0.6239 | 0.6574 | 0.5337 | 0.5457 | 0.5641 | 0.5865 | 0.504 | 0.5104 | 0.5207 | 0.5341 |
| | 5 | 0.5821 | 0.6191 | 0.6611 | 0.6908 | 0.5559 | 0.5663 | 0.5824 | 0.6024 | 0.531 | 0.5355 | 0.5427 | 0.5524 | 0.5026 | 0.505 | 0.5088 | 0.5141 |
| | 10 | 0.5712 | 0.5821 | 0.5987 | 0.6191 | 0.5532 | 0.5559 | 0.5603 | 0.5663 | 0.5298 | 0.531 | 0.5329 | 0.5355 | 0.502 | 0.5026 | 0.5036 | 0.505 |
| 0.15 | 1 | 0.6814 | 0.594 | 0.5168 | 0.4572 | 0.594 | 0.627 | 0.5639 | 0.4903 | 0.5168 | 0.5639 | 0.5596 | 0.5093 | 0.4572 | 0.4903 | 0.5093 | 0.4937 |
| | 1.5 | 0.6647 | 0.6834 | 0.6451 | 0.6183 | 0.5638 | 0.6283 | 0.662 | 0.6593 | 0.5014 | 0.5376 | 0.5784 | 0.6075 | 0.4489 | 0.4688 | 0.4962 | 0.5245 |
| | 2 | 0.6333 | 0.6967 | 0.6846 | 0.6642 | 0.5497 | 0.5978 | 0.6454 | 0.6738 | 0.4952 | 0.518 | 0.5495 | 0.5827 | 0.4458 | 0.4576 | 0.4757 | 0.4978 |
| | 2.5 | 0.6119 | 0.6848 | 0.6992 | 0.6859 | 0.5427 | 0.5771 | 0.6192 | 0.6553 | 0.4923 | 0.5075 | 0.5301 | 0.557 | 0.4443 | 0.452 | 0.4643 | 0.4801 |
| | 3 | 0.5982 | 0.6663 | 0.6995 | 0.6979 | 0.5388 | 0.5641 | 0.5984 | 0.6331 | 0.4907 | 0.5015 | 0.5181 | 0.539 | 0.4435 | 0.4489 | 0.4577 | 0.4693 |
| | 5 | 0.5754 | 0.612 | 0.6543 | 0.686 | 0.5329 | 0.5427 | 0.558 | 0.5773 | 0.4883 | 0.4923 | 0.4988 | 0.5075 | 0.4423 | 0.4443 | 0.4476 | 0.4521 |
| | 10 | 0.5647 | 0.5754 | 0.5918 | 0.612 | 0.5303 | 0.5329 | 0.537 | 0.5427 | 0.4873 | 0.4883 | 0.49 | 0.4923 | 0.4418 | 0.4423 | 0.4432 | 0.4443 |
| 0.2 | 1 | 0.666 | 0.561 | 0.4676 | 0.398 | 0.561 | 0.5825 | 0.5065 | 0.4251 | 0.4676 | 0.5065 | 0.4937 | 0.4378 | 0.398 | 0.4251 | 0.4378 | 0.4182 |
| | 1.5 | 0.6547 | 0.6777 | 0.6404 | 0.6102 | 0.5337 | 0.5959 | 0.6322 | 0.6324 | 0.4543 | 0.4866 | 0.5245 | 0.5536 | 0.3912 | 0.408 | 0.4316 | 0.4569 |
| | 2 | 0.6235 | 0.6911 | 0.6867 | 0.6713 | 0.5206 | 0.5664 | 0.6149 | 0.6485 | 0.4489 | 0.4691 | 0.4978 | 0.5295 | 0.3886 | 0.3985 | 0.4139 | 0.4332 |
| | 2.5 | 0.6023 | 0.6771 | 0.6988 | 0.693 | 0.5141 | 0.5465 | 0.5879 | 0.6264 | 0.4464 | 0.4597 | 0.4801 | 0.5049 | 0.3873 | 0.3938 | 0.4042 | 0.4178 |



| | | | | | | | | | | | | | | | | |
|----|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| 3 | 0.5887 | 0.6574 | 0.6957 | 0.7015 | 0.5104 | 0.5341 | 0.5672 | 0.6024 | 0.4449 | 0.4544 | 0.4693 | 0.4882 | 0.3866 | 0.3912 | 0.3986 | 0.4084 |
| 5 | 0.5663 | 0.6024 | 0.6451 | 0.679 | 0.505 | 0.5141 | 0.5285 | 0.5468 | 0.4429 | 0.4464 | 0.4521 | 0.4598 | 0.3857 | 0.3873 | 0.3901 | 0.3938 |
| 10 | 0.5559 | 0.5663 | 0.5825 | 0.6024 | 0.5026 | 0.505 | 0.5088 | 0.5141 | 0.442 | 0.4429 | 0.4443 | 0.4464 | 0.3852 | 0.3857 | 0.3864 | 0.3873 |

REFERENCES

- [1]. R. D. Mindlin. Influence of rotary inertia and shear on flexural motions of isotropic, elastic plates. *Journal of Applied Mechanics*, 18(1): 31-38, 1951. doi: 10.1115/1.4010217.
- [2]. A. W. Leissa. *Vibration of Plates (NASA SP-160)*. Washington DC: U.S. Government Printing Office, 1969.
- [3]. A. W. Leissa. The free vibration of rectangular plates. *Journal of Sound and Vibration*, 31(3): 257-293, 1973. doi: 10.1016/S0022-460X(73)80371-2.
- [4]. G. V. Rao, K. Meerasaheb and G. Rangajanardha. Concept of coupled displacement field for large amplitude free vibrations of shear flexible beams. *American Society of Mechanical Engineering*, 128(2): 251-255, 2006. doi:10.1115/1.2159038
- [5]. C. M. Wang, C. Y. Wang, and J. N. Reddy. *Exact solutions for buckling of structural members*. CRC Press, 2005.
- [6]. M. Batista, Analytical solution for free vibrations of simply supported transversally inextensible homogeneous rectangular plate, *physics.gen-ph*, 2010. doi: 10.48550/arXiv.1007.2539.
- [7]. S. H. Hashemi, and M. Arsanjani, Exact characteristic equations for some of classical boundary conditions of vibrating moderately thick rectangular plates, *International Journal of Solids and Structures*, 42 (3-4): 819-853, 2005. doi: 10.1016/j.ijsolstr.2004.06.063.
- [8]. K. K Pradhan, and S. Chakraverty. Transverse vibration of isotropic thick rectangular plates based on new inverse trigonometric shear deformation theories. *International Journal of Mechanical Sciences*, 94-95: 211-231, 2015. doi: 10.1016/j.ijmecsci.2015.03.009.
- [9]. R. P. Shimpi, H. G. Patel, and H. Arya. New first-order shear deformation plate theories. *Journal of Applied Mechanics*, 74: 523-533, 2007.
- [10]. H. T. Thai and D. H. Choi. Analytical solutions of refined plate theory for bending, buckling and vibration analyses of thick plates. *Applied Mathematical Modelling*, 37(18-19): 8310-8323, 2013. doi: 10.1016/j.apm.2013.03.038.
- [11]. A. Mahi, El Abbas , A. Tounsi and A. Benkhedda, A new simple shear deformation theory for free vibration analysis of isotropic and FG plates under different boundary conditions. *Multidiscipline Modeling in Materials and Structures*, 11(3): 437-470, 2015. doi: 10.1108/MMMS-03-2015-0010.
- [12]. A. Mahi, El Abbas , A. Tounsi, and A. Benkhedda. A new hyperbolic shear deformation theory for bending and free vibration analysis of isotropic, functionally graded, sandwich and laminated composite plates. *Applied Mathematical Modelling*, 39(9): 2489-2508, 2015. doi: 10.1016/j.apm.2014.10.045.
- [13]. A. S. Sayyada, and Y. M. Ghugal. Bending and free vibration analysis of thick isotropic plates by using exponential shear deformation theory. *Applied and Computational Mechanics*, 6(1): 65- 82, 2012. doi: 10.1016/j.compositesb.2012.01.062.
- [14]. A. M. A. Neves, A. J. M. Ferreira, E. Carrera, M. Cinefra, C. M. C. Roque, R. M. N. Jorge, and C.M.M. Soares. Static, free vibration and buckling analysis of isotropic and sandwich functionally graded plates using a quasi-3D higher-order shear deformation theory and a meshless technique, *Composites: Part B*, 44(1): 657-674, 2013. doi: 10.1016/j.compositesb.2012.01.089
- [15]. M. Batista. Analytical solution for free vibrations of simply supported transversally inextensible homogeneous rectangular plate. *arXiv:1007.2539 [physics.gen-ph]*.
- [16]. K. M. Liew, Y. Xiang, and S. Kitipornchai. Transverse vibration of thick rectangular plates-I. Comprehensive sets of boundary conditions. *Computers & Structures*, 49(1): 1-29, 1993. doi: 10.1016/0045-7949(93)90122-T.
- [17]. S. H. Hashemi, M. Fadaee, and H. R. D. Taher. Exact solutions for free flexural vibration of Levy-type rectangular thick plates via third-order shear deformation plate theory. *Applied Mathematical Modelling*, 35(2): 708-727, 2011. doi: 10.1016/j.apm.2010.07.028.
- [18]. D. Shi, Q. Wang, X. Shi, and F. Pang. Free vibration analysis of moderately thick rectangular plates with variable thickness and arbitrary boundary conditions. *Shock and Vibration*, 2014(1): 1-25, 2014. doi: 10.1155/2014/572395.
- [19]. K. K. Raju, G. V. Rao, and I. S. Raju. Effect of geometric nonlinearity on large amplitude free flexural vibrations of moderately thick rectangular plates. *Computers and Structures*, 9(5): 441-444, 1978. doi: 10.1016/0045-7949(78)90040-8.
- [20]. K. K. Raju, and E. Hinton. Natural frequencies and modes of rhombic Mindlin plates, *Earthquake Engineering Structural Dynamics*, 8 (1): 55-62, 1980. doi: 10.1002/eqe.4290080106.
- [21]. B. S. Sarma. Nonlinear free vibrations of beams, plates, and nonlinear panel flutter. *Ph.D. thesis, Department of Aerospace Engineering, I.I.T., Madras*, 1987.
- [22]. K. K. Raju, and E. Hinton. Nonlinear vibrations of thick plates using Mindlin plate elements. *International Journal of Numerical Methods in Engineering*, 15(2): 241-257, 1980. doi: 10.1002/nme.1620150208.
- [23]. C. Mei, and K. Decha-Umphai, A finite element method for nonlinear forced vibrations of rectangular plates. *AIAA Journal*, 23(7): 1104-1110, 1985. doi: 10.2514/3.9044.
- [24]. Y. Shi, and C. Mei. A finite element time domain modal formulation for large amplitude free vibrations of beams and plates. *Journal of Sound and Vibration*, 193(2): 453-464, 1996. doi: 10.1006/jsvi.1996.0295.



- [25]. T. Wah. Large amplitude flexural vibration of rectangular plates. *International Journal of Mechanical Sciences*, 5(6): 425-438, 1963. doi: 10.1016/0020-7403(63)90026-2.
- [26]. H. N. Chu, and Herrman. G. Influence of large amplitudes on free flexural vibrations of rectangular elastic plates. *Journal of Applied Mechanics*, 23(4): 532-540, 1956. doi: 10.1115/1.4011396.
- [27]. C. Mei. Finite element displacement method for large amplitude free flexural vibrations of beams and plates, *Computers & Structures*, 3(1): 163-174, 1973. doi: 10.1016/0045-7949(73)90081-